Bryan W. Shaw, Ph.D., Chairman Buddy Garcia, Commissioner Carlos Rubinstein, Commissioner Mark R. Vickery, P.G., Executive Director

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Protecting Texas by Reducing and Preventing Pollution

November 17, 2010

RECEIVED

NOV 2 3 2010

Mr. J. L. Guerra, Jr. Westpointe Residential, Ltd. PO Box 212 Colonial Heights, VA, 23834

COUNTY ENGINEER

Re: Edwards Aquifer, Comal County

NAME OF PROJECT: The Enclave at Westpointe Village; Located at the southwest corner of Oak Run Pkwy and Independence Drive, New Braunfels, Texas TYPE OF PLAN: Request for the Approval of a Water Pollution Abatement Plan (WPAP); 30 Texas Administrative Code (TAC) Chapter 213 Edwards Aquifer San Antonio File No. 2942.00; Investigation No. 858288; Regulated Entity No. RN105991145

Dear Mr. Guerra:

The Texas Commission on Environmental Quality (TCEQ) has completed its review of the WPAP application for the above-referenced project submitted to the San Antonio Regional Office by Bury+Partners on behalf of Westpointe Residential, Ltd. on August 23, 2010. Final review of the WPAP was completed after additional material was received on October 15, 2010. November 12, 2010 and November 16, 2010. As presented to the TCEQ, the Temporary and Permanent Best Management Practices (BMPs) and construction plans were prepared by a Texas Licensed Professional Engineer to be in general compliance with the requirements of 30 TAC Chapter 213. These planning materials were sealed, signed and dated by a Texas Licensed Professional Engineer. Therefore, based on the engineer's concurrence of compliance, the planning materials for construction of the proposed project and pollution abatement measures are hereby approved subject to applicable state rules and the conditions in this letter. The applicant or a person affected may file with the chief clerk a motion for reconsideration of the executive director's final action on this Edwards Aguifer Protection Plan. A motion for reconsideration must be filed no later than 23 days after the date of this approval letter. This approval expires two (2) years from the date of this letter unless, prior to the expiration date, more than 10 percent of the construction has commenced on the project or an extension of time has been requested.

PROJECT DESCRIPTION

The proposed residential project will have an area of approximately 48.18 acres. It will include the construction of 134 residential houses, three sedimentation filtration basins, a parkland/amenity center and associated roadways and sidewalks. The impervious cover will be 22.63 acres (46.97 percent). Project wastewater will be disposed of by conveyance to the existing Gruene Water Recycling Center owned by New Braunfels Utility.

PERMANENT POLLUTION ABATEMENT MEASURES

To prevent the pollution of stormwater runoff originating on-site or upgradient of the site and potentially flowing across and off the site after construction, three sand filter basins, designed using the TCEQ technical guidance document, <u>Complying with the Edwards Aquifer Rules: Technical Guidance on Best Management Practices</u> (RG-348, 2005), will be constructed to treat stormwater runoff. The required total suspended solids (TSS) treatment for this project is 20,313 pounds of TSS generated from the 22.63 acres of impervious cover. The approved measures meet the required 80 percent removal of the increased load in TSS caused by the project.

The individual treatment measures will consist of three single-chamber, earthen sand filter basins. Each basin will have a 12 inch clay liner and three to one side slopes. The filtration system will consists of an 18 inch sand layer separated from the eight inch gravel layer by a geotextile fabric layer. The 4 inch underdrain pipes will be placed on the bottom of the basins. The required and the designed water quality volumes, sand filter areas and TSS removal amounts for each basin are summarized in table below.

	Table	1: Perma	nent BMP7	rss Removal	and Sizin	g Summa	ry Table	
Drainage Area	Total Area (ac)	Imp. Cover (ac)	Req. TSS Removal (lb/yr)	Design TSS Removal (lb/yr)	Req. WQV (ft³)	Design WQV (ft³)	Req. Sand Filter Area (ft²)	Design Sand Filter Area (ft³)
DA-1 West Basin	12.59	7.35	6,597	6,910	61,065	63,830	4,039	18,310
DA -2 East Basin	19.68	10.25	9,200	9,775	63,332	65,370	6,334	17,401
DA-3 SW Basin	6.24	3.55	3,186	3,630	39,703	50,272	3,971	7,366
Uncaptured	3.91	1.48	1,328	0				
Total	42.42	22.63	20,313	20,315				

GEOLOGY

According to the geologic assessment included with the application, three geologic features were located at the site. Two features were evaluated as non-sensitive by the project geologist. One feature was ranked as sensitive and is further discussed below. The San Antonio Regional Office conducted a site assessment on September 27, 2010, which revealed the site was adequately described in the geologic assessment.

Sensitive Features

A natural buffer was proposed for one sensitive solution cavity (F-2). No regulated activities (such as construction or soil disturbing activities) will take place within the natural buffer area. In accordance with Ch. 5 of RG-348, the size of the natural buffer is based on the size of the feature and on the drainage area to the sensitive feature. The buffer will extend a minimum of 50 feet in all directions from the edge of the two foot by one foot feature and will extend 100 feet

Mr. J.L. Guerra, Jr. November 17, 2010 Page 3

upgradient of the feature, to the edge of the feature's drainage area. The natural buffer layout is detailed and labeled on the WPAP site plan and exhibits.

SPECIAL CONDITIONS

- I. The permanent BMP shall be operational prior to occupancy of any residential house lots, occupancy of any sales office (model homes) or public use of the roadways within the respective BMP's drainage area.
- II. All sediment and/or media removed from the permanent BMPs during maintenance activities shall be properly disposed of according to 30 TAC 330 or 30 TAC 335, as applicable.
- III. Any change in the design, specifications, size or layout of the permanent BMPs or any change in the approved regulated activities that would impact the ability of the temporary and permanent BMPs to prevent pollution of the Edwards Aquifer may require prior approval of a WPAP modification.

STANDARD CONDITIONS

- 1. Pursuant to Chapter 7 Subchapter C of the Texas Water Code, any violations of the requirements in 30 TAC Chapter 213 may result in administrative penalties.
- 2. The holder of the approved Edwards Aquifer protection plan must comply with all provisions of 30 TAC Chapter 213 and all best management practices and measures contained in the approved plan. Additional and separate approvals, permits, registrations and/or authorizations from other TCEQ Programs (i.e., Stormwater, Water Rights, UIC) can be required depending on the specifics of the plan.
- 3. In addition to the rules of the Commission, the applicant may also be required to comply with state and local ordinances and regulations providing for the protection of water quality.

<u>Prior to Commencement of Construction</u>:

- 4. Within 60 days of receiving written approval of an Edwards Aquifer Protection Plan, the applicant must submit to the San Antonio Regional Office, proof of recordation of notice in the county deed records, with the volume and page number(s) of the county deed records of the county in which the property is located. A description of the property boundaries shall be included in the deed recordation in the county deed records. A suggested form (Deed Recordation Affidavit, TCEQ-0625) that you may use to deed record the approved WPAP is enclosed.
- 5. All contractors conducting regulated activities at the referenced project location shall be provided a copy of this notice of approval. At least one complete copy of the approved WPAP and this notice of approval shall be maintained at the project location until all regulated activities are completed.

- 6. Modification to the activities described in the referenced WPAP application following the date of approval may require the submittal of a plan to modify this approval, including the payment of appropriate fees and all information necessary for its review and approval prior to initiating construction of the modifications.
- 7. The applicant must provide written notification of intent to commence construction, replacement, or rehabilitation of the referenced project. Notification must be submitted to the San Antonio Regional Office no later than 48 hours prior to commencement of the regulated activity. Written notification must include the date on which the regulated activity will commence, the name of the approved plan and program ID number for the regulated activity, and the name of the prime contractor with the name and telephone number of the contact person. The executive director will use the notification to determine if the approved plan is eligible for an extension.
- 8. Temporary erosion and sedimentation (E&S) controls, i.e., silt fences, rock berms, stabilized construction entrances, or other controls described in the approved WPAP, must be installed prior to construction and maintained during construction. Temporary E&S controls may be removed when vegetation is established and the construction area is stabilized. If a water quality pond is proposed, it shall be used as a sedimentation basin during construction. The TCEQ may monitor stormwater discharges from the site to evaluate the adequacy of temporary E&S control measures. Additional controls may be necessary if excessive solids are being discharged from the site.
- 9. All borings with depths greater than or equal to 20 feet must be plugged with non-shrink grout from the bottom of the hole to within three (3) feet of the surface. The remainder of the hole must be backfilled with cuttings from the boring. All borings less than 20 feet must be backfilled with cuttings from the boring. All borings must be backfilled or plugged within four (4) days of completion of the drilling operation. Voids may be filled with gravel.

During Construction:

- During the course of regulated activities related to this project, the applicant or agent shall comply with all applicable provisions of 30 TAC Chapter 213, Edwards Aquifer. The applicant shall remain responsible for the provisions and conditions of this approval until such responsibility is legally transferred to another person or entity.
- This approval does not authorize the installation of temporary aboveground storage tanks on this project. If the contractor desires to install a temporary aboveground storage tank for use during construction, an application to modify this approval must be submitted and approved prior to installation. The application must include information related to tank location and spill containment. Refer to Standard Condition No. 6, above.
- If any sensitive feature (caves, solution cavities, sink holes, etc.) is discovered during construction, all regulated activities near the feature must be suspended immediately. The applicant or his agent must immediately notify the San Antonio Regional Office of

the discovery of the feature. Regulated activities near the feature may not proceed until the executive director has reviewed and approved the methods proposed to protect the feature and the aquifer from potentially adverse impacts to water quality. The plan must be sealed, signed, and dated by a Texas Licensed Professional Engineer.

- No wells are located on the site. All water wells, including injection, dewatering, and monitoring wells must be in compliance with the requirements of the Texas Department of Licensing and Regulation under Title 16 TAC Chapter 76 (relating to Water Well Drillers and Pump Installers) and all other locally applicable rules, as appropriate.
- 14. If sediment escapes the construction site, the sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain). Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50 percent. Litter, construction debris, and construction chemicals shall be prevented from becoming stormwater discharge pollutants.
- 15. Intentional discharges of sediment laden storm water are not allowed. If dewatering becomes necessary, the discharge will be filtered through appropriately selected best management practices. These may include vegetated filter strips, sediment traps, rock berms, silt fence rings, etc.
- 16. The following records shall be maintained and made available to the executive director upon request: the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 17. Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, and construction activities will not resume within 21 days. When the initiation of stabilization measures by the 14th day is precluded by weather conditions, stabilization measures shall be initiated as soon as practicable.

After Completion of Construction:

- 18. A Texas Licensed Professional Engineer must certify in writing that the permanent BMPs or measures were constructed as designed. The certification letter must be submitted to the San Antonio Regional Office within 30 days of site completion.
- The applicant shall be responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. The regulated entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred. A copy of the transfer of responsibility must be filed with the executive director through San Antonio Regional Office within 30 days of the transfer. A copy of the transfer form (TCEQ-10263) is enclosed.

- 20. Upon legal transfer of this property, the new owner(s) is required to comply with all terms of the approved Edwards Aquifer protection plan. If the new owner intends to commence any new regulated activity on the site, a new Edwards Aquifer protection plan that specifically addresses the new activity must be submitted to the executive director. Approval of the plan for the new regulated activity by the executive director is required prior to commencement of the new regulated activity.
- 21. An Edwards Aquifer protection plan approval or extension will expire and no extension will be granted if more than 50 percent of the total construction has not been completed within ten years from the initial approval of a plan. A new Edwards Aquifer protection plan must be submitted to the San Antonio Regional Office with the appropriate fees for review and approval by the executive director prior to commencing any additional regulated activities.
- 22. At project locations where construction is initiated and abandoned, or not completed, the site shall be returned to a condition such that the aquifer is protected from potential contamination.

If you have any questions or require additional information, please contact Charly Fritz of the Edwards Aquifer Protection Program of the San Antonio Regional Office at (210) 403-4065.

Sincerely,

Mark R. Vickery, P.G.

Executive Director

Texas Commission on Environmental Quality

MRV/CEF/eg

Enclosures: Deed Recordation Affidavit, Form TCEQ-0625

Change in Responsibility for Maintenance of Permanent BMPs, Form TCEQ-

10263

cc: Mr. Aaron Parenica, P.E., Bury+Partners

Mr. Wesley Hamff, New Braunfels Utilities

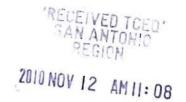
Mr. Thomas Hornseth, P.E., Comal County Engineer

Mr. Karl Dreher, General Manager, Edwards Aquifer Authority

TCEQ Central Records, Building F, MC 212



November 12, 2010



Project No.: R0101769-50001.61

Ms. Charly Fritz
San Antonio – Edwards Aquifer
Protection Program
Texas Commission on Environmental Quality
14250 Judson Road
San Antonio, Texas 78233

Re: Edwards Aquifer, Comal County

NAME OF PROJECT: The Enclave at Westpointe Village: (WPAP); Located at the SW corner of Oak Run Pkwy and Independence Dr., New Braunfels, Texas PLAN TYPE: Request for the Approval to an Approved Water Pollution Abatement Plan (WPAP); 30 Texas Administrative Code (TAC) Chapter 213;

San Antonio File No. 2942.00

Dear Ms. Fritz:

The following is our response to the review comments received from your office dated November 1, 2010.

General

1. Comment: The response to the first NOD stated each drainage area will have approximately 0.50 acres of impervious cover associated to parks, playgrounds or amenity centers. However, for a WPAP to be approved, the plan must show all lots, recreation centers, buildings, roads and sidewalks, must provide the amount of impervious cover and must detail the drainage from these areas. Please indicate the location or locations of any parks, playgrounds or amenity centers on the site plan and provide the layout of these items with the amount of impervious cover and the direction of storm water flow.

BURY+PARTNERS-SA, INC. 922 Isom Road, Suite 100 San Antonio, Texas 78216



Response: Per our conversation on November 9, 2010, the WPAP Site Plan shows all lots and roads that are proposed for this subdivision that will be constructed by the developer. However, actual sidewalk placement, house footprints, and amenities (including clubhouse) will be constructed by the homebuilder. Those plans have not been developed at this stage of the design process. Therefore, careful design considerations, such as additional impervious area, as well as the TCEQ Technical Guidance Manual, have been incorporated in this plan to account for such future building pads and amenities. Furthermore, as requested, parkland areas have been labeled on the Drainage Area Map, as well as the assigned lot for the location of the Recreational Center.

2. Comment: For the WPAP site plan, please remove the utility lines and labels (water, sewer) to provide a clearer map.

Response: The utility lines for water, sewer, and proposed force main have been removed to provide a clearer map.

3. Comment: Provide elevation labels for all final contours.

Response: Elevation labels for final contours have been provided.

4. Comment: Do not include the water quality basins or detention basins with the drainage areas for the water quality basins. The drainage area for a basin should reflect the amount of previous and impervious cover captured by that basin. Update the calculations as necessary.

Response: The drainage areas have been revised to exclude the areas encompassing the detention and water quality basins. The calculations have been updated on the WPAP Calculation Sheet.

5. Comment: Label any uncaptured areas on the drainage area map and update. Any uncaptured areas should not be included within the basin drainage areas. If impervious cover is uncaptured and untreated, the TSS amount from that impervious cover can be accounted for in a basin. Increasing the TSS amount treated by the basin in Step 5 of the sizing calculation spreadsheet.

Response: There is a 3.91-acre area on the northwest corner of the project site, adjacent to the existing residential neighborhood that will not be treated. The imperious coverage in this area has been added to the Summary Tables on the WPAP Calculation Sheet. The TSS load and volume requirement for this area has been accounted for with the over treatment provided by the three water quality ponds, as well as the increase in the desired TSS treatment in Step 5 of the TCEQ Calculation Spreadsheet for each pond. The surplus load treatment and volume is shown to be greater than the volume and load removal required by this 3.91 acre portion of the project site.



6. Comment: Review the drainage area map. There appear to be areas included in a drainage area that either drain to adjacent drainage area or are uncaptured, more specifically, contour line sand flows do not agree with drainage boundary lines. Please revise the map and basin sizing calculation as necessary.

Proposed Westpointe Drive:

- a) Based upon contours, there appears to be storm water flowing onto Westpointe Drive from Oak Run Pkwy. As part of the original Westpointe development, TSS from this parkway is treated by the existing sand filter basin at the northern corner of the site. Either revise the intersection design to have storm water flow, from the parkway, continue to the existing sand filter basin or account for the parkway TSS in the WPAP.
- b) At the boundary between DA-1 and DA-2, there appears to be storm water flowing along the roadway from DA-2 into DA-1.
- c) At the boundary between DA-1 and DA-3, there appears to be storm water from DA-3 that flows into DA-1 and therefore, is not captured by the SW basin.
- d) For DA-3, the roadway appears to be at a lower elevation than the inlet to the basin and storm drains are not provided.

Response: There will be water flowing from Oak Run Parkway onto Westpointe Drive. Drainage Area 2 (DA-2) has been revised to reflect the additional drainage from Oak Run Parkway. Also, the contours between DA-1 and DA-2 do depict cross flow between the two drainage areas. The future addition of two curb inlets at this junction will prevent runoff from DA-2 from entering into DA-1. These inlets are not shown as the plans for this segment of Westpointe Drive have not been designed. Such plans will be part of Phase II of the project. As noted during our meeting on November 9, 2010, a future pond system will be connected in series to a pipe leading to the proposed East Water Quality Pond. As discussed, the before-mentioned inlets will connect to that pipe. Please note that the pond system is designed such that the water upgradient from this drainage basin would be treated and detained by a future water quality and detention pond prior to being conveyed through the pipe. In addition, the boundary between DA-1 and DA-3 has been revised to match the proposed high point along Westpointe Drive; however, we have removed a portion of the roadway from DA-3. Once this segment of Westpointe Drive is designed in the future, a separate WPAP will have to be submitted for this segment of the roadway as well as the portions of the adjacent tract to the south that are not included in this application.



DA-1:

- a) The boundary between DA-1 and DA-3, in the western corner of the site, does not appear to be at the high point based upon contour lines.
- b) The house lots outside the internal roadway (i.e., that backup to the existing neighborhood) do not appear to drain towards the roadway and therefore, might not be captured by the basin.

Response: The boundary between DA-1 and DA-3 at the western corner of the site has been revised to match the high points shown. Furthermore, the house lots on the outside of the internal roadway are now labeled as an area not captured. The volume and load removal requirements are satisfied through overtreatment and capture by the proposed water quality ponds. Calculations are provided and shown on the WPAP Calculation Sheet.

DA-3:

- a) The house lots outside the internal roadway do not appear to drain towards the roadway and therefore, might not be captured by the basin.
- b) As state above under Westpointe Drive item, the roadway and the upgradient drainage area appears to be at a lower elevation than the basin inlet and will not be captured by the basin.

Response: As previously mentioned, the house lots on the outside of the internal roadway are now part of and labeled as an area not being captured. The volume and load removal requirements are satisfied through overtreatment and capture by the proposed water quality ponds. Furthermore, the westernmost segment of Westpointe Drive has been removed from DA-2 and will be treated by a separate WPAP submittal once design and development of this area commences.

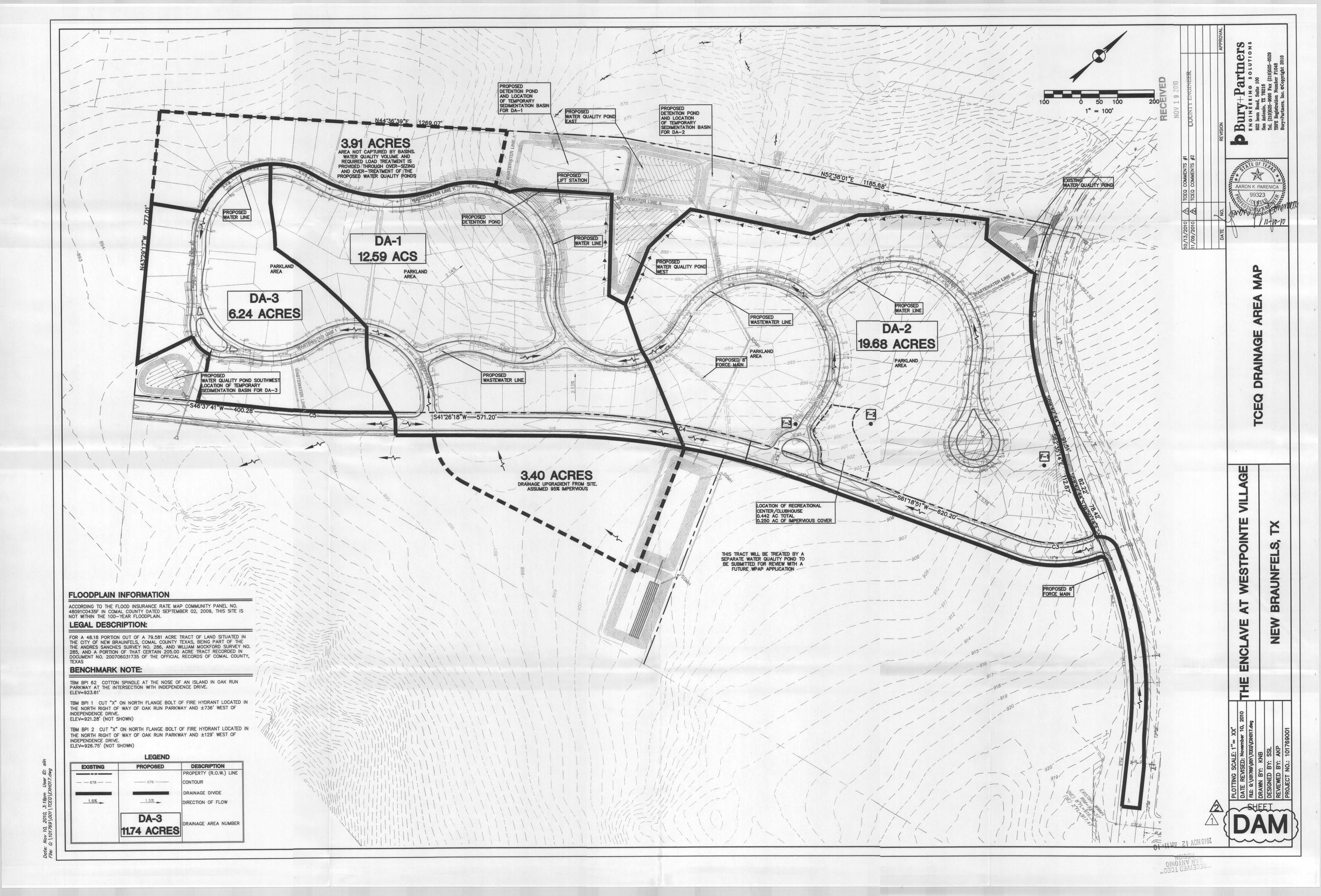
If you have further questions or need additional information, please do not hesitate to contact me.

Sincerely,

Aaron K. Parenica, P.E. Associate/Project Manager

Bury + Partners-SA, Inc. TBPE F-1048

Attachments



TSS Removal Calculations		
Project Name: The Enclave at Westpointe Village Date Prepared: 11/8/2010		
1. The Required Load Reduction for the total project:		
Calculations from RG-348 Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$		
Pages 3-27 to 3-30 $L_{M TOTAL \ PROJECT} = Required \ TSS \ removal \ resulting \ from \ the \ proposed \ development = 80\%$	6 of increase	ed load
A_N = Net increase in impervious area for the project P = Average annual precipitation, inches		
Site Data: Determine Required Load Removal Based on the Entire Project County =	Comal	
Total project area included in plan * =	38.51	acres
Predevelopment impervious area within the limits of the plan * = Total post-development impervious area within the limits of the plan* =	0.00 21.15	acres
Total post-development impervious cover fraction * = P =	0.55	inches
$L_{M TOTAL PROJECT} =$	18984	lbs.
Number of drainage basins / outfalls areas leaving the plan area =	3	
2. Drainage Basin Parameters (This information should be provided for each basin):		
Drainage Basin/Outfall Area No. =	2	
Total drainage basin/outfall area =	19.68	acres
Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area = Post-development impervious fraction within drainage basin/outfall area =	10.25 0.52	acres
$L_{M \text{ THIS BASIN}} =$	9205	lbs.
3. Indicate the proposed BMP Code for this basin.		
Proposed BMP =	SF	abbreviati
Removal efficiency =	89	percent
4. Calculate Maximum TSS Load Removed (L _R) for this Drainage Basin by the selected BMP Type.		
RG-348 Page 3-33 Equation 3.7:		
LR = (BMP efficiency) x P x (A_1 x 34.6 + A_P x 0.54)		
A _C = Total On-Site drainage area in the BMP catchment area		
A_I = Impervious area proposed in the BMP catchment area A_P = Pervious area remaining in the BMP catchment area		
L_R = TSS Load removed from this catchment area by the proposed BMP		
$A_{C}=$	19.68	acres
$A_{I} = A_{P} = A_{P}$	10.25 9.43	acres
$L_{R}=% \frac{1}{2} L_{R}=% \frac{1}$	10571	lbs
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area $Desired\ L_{M\ THIS\ BASIN} =$	9775	lbs
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area		
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5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348	9775 0.92	lbs.
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth =	9775	
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348	9775 0.92	lbs.
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348	9775 0.92 2.00 0.37 52777	lbs.
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP =	9775 0.92 2.00 0.37 52777	lbs. inches cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area =	9775 0.92 2.00 0.37 52777 0.00 0.00 0	lbs. inches cubic fee
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP =	9775 0.92 2.00 0.37 52777 0.00 0.00	inches cubic fee
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume =	9775 0.92 2.00 0.37 52777 0.00 0.00 0	inches cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient =	9775 0.92 2.00 0.37 52777 0.00 0.00 0	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	9775 0.92 2.00 0.37 52777 0.00 0.00 0	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0	inches cubic feet acres cubic feet cubic feet
Desired L _{M This Basin} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Required Water Quality Volume for retention basin = Pages 3-42 to 3-46	9775 0.92 2.00 0.37 52777 0.00 0.00 0	inches cubic feet acres cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Required Water Quality Volume for retention basin =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0	inches cubic feet acres cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Off-site Water Quality Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332	inches cubic feet acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332	inches cubic feet acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Off-site Water Quality Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Required Water Quality Volume for extended detention basin =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres cubic feet cubic feet cubic feet cubic feet
Desired L _{M This Basin} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres cubic feet cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Required Water Quality Volume for extended detention basin =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 P. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-46 to 3-51	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres cubic feet cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 P. Filter area for Sand Filters	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soll infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA	inches cubic feet acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Minimum filter basin area =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA	inches cubic fee acres cubic fee
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area a Off-site Water Quality Volume = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 10555 63332 NA NA NA	inches cubic feet acres acres cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for extended detention basin = Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA NA	inches cubic feet acres acres cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious fraction of off-site area = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for extended detention basin = Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA NA	inches cubic feet acres acres cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Off-site Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 P. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum sedimentation basin area (2' Depth) =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA NA	inches cubic feet acres cubic feet square fe square fe square fe
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Maximum sedimentation basin area (2' Depth) = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum sedimentation basin area (2' Depth) =	9775 0.92 2.00 0.37 52777 0.00 0.00 0 0.00 0 10555 63332 NA NA NA NA NA NA	inches cubic feet acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious cover draining to BMP = Off-site Mater Quality Volume = Storage for Sediment = Off-site Water Quality Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) =	9775 0.92 2.00 0.37 52777 0.00 0 0.00 0 10555 63332 NA NA NA NA NA	inches cubic feet acres cubic feet cubic feet cubic feet

TSS Removal Calculations	ental Quality		
Project Name Date Prepared	: The Enclave at Westpointe Village : 11/8/2010		
1. The Required Load Reduction	for the total project:		
Calculations from RG-348 Pages 3-27 to 3-30	Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$		
$A_N =$	= Required TSS removal resulting from the proposed development = 80 = Net increase in impervious area for the project = Average annual precipitation, inches	% of increase	ed load
Site Data	: Determine Required Load Removal Based on the Entire Project		
	County = Total project area included in plan * =	Comal 38.51	acres
	Predevelopment impervious area within the limits of the plan * =	0.00	acres
	Total post-development impervious area within the limits of the plan* = Total post-development impervious cover fraction * =	0.55	acres
	P =		inches
	$L_{M TOTAL PROJECT} =$	18984	lbs.
	Number of drainage basins / outfalls areas leaving the plan area =	3	
2. Drainage Basin Parameters (T	This information should be provided for each basin):		
	Drainage Basin/Outfall Area No. =	3	
	Total drainage basin/outfall area =	6.24	acres
	Predevelopment impervious area within drainage basin/outfall area = Post-development impervious area within drainage basin/outfall area =	0.00	acres
Pos	st-development impervious fraction within drainage basin/outfall area =	0.57	
	$L_{M \text{ THIS BASIN}} =$	3188	lbs.
3. Indicate the proposed BMP C	ode for this basin.		
	Proposed BMP = Removal efficiency =		abbreviation percent
4 Calculate Maximum TCC I			percent
4. Calculate Maximum TSS Loa	d Removed (L _R) for this Drainage Basin by the selected BMP Type.		
	RG-348 Page 3-33 Equation 3.7: $LR = (BMP \text{ efficiency}) \times P \times (A_I \times 34.6 + A_P \times 0.54)$		
A _C =	= Total On-Site drainage area in the BMP catchment area		
$A_{I} =$	= Impervious area proposed in the BMP catchment area		
	 Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed BMP 		
	$A_{C} =$	6.24	acres
	$A_{\rm I} =$		acres
	$A_{P} = L_{R} =$	2.69	acres
5 Colculate Fraction of Annual	$A_P = L_R =$	2.69	acres
5. Calculate Fraction of Annual	$A_{P} = \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area$	2.69 3652	acres lbs
5. Calculate Fraction of Annual	$A_P = L_R =$	2.69 3652 3630	acres
	$$A_P=$L_R=$ $L_R=$$ Runoff to Treat the drainage basin / outfall area ${\rm Desired}\; L_{M\;THIS\; BASIN}=$ $F=$	2.69 3652 3630	acres lbs
6. Calculate Capture Volume re	$A_P = \\ L_R = \\ Runoff to Treat the drainage basin / outfall area \\ Desired L_{M THIS BASIN} = \\ Constant Control of the property of the pro$	2.69 3652 3630	acres lbs
	$A_P = \\ L_R = \\ Runoff to Treat the drainage basin / outfall area \\ Desired L_{M THIS BASIN} = \\ F = \\ quired by the BMP Type for this drainage basin / outfall area. \\ Rainfall Depth = \\ Comparison of the property of the$	2.69 3652 3630 0.99	acres lbs
6. Calculate Capture Volume recalculations from RG-348	$A_P = \\ L_R = \\ Runoff to Treat the drainage basin / outfall area$ $Desired \ L_{M \ THIS \ BASIN} = \\ F = \\ Quired \ by \ the \ BMP \ Type \ for \ this \ drainage \ basin / outfall \ area.$	2.69 3652 3630 0.99	acres lbs
6. Calculate Capture Volume recalculations from RG-348	$A_{P} = \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area \\ \\ Desired L_{M THIS BASIN} = \\ F = \\ \\ Quired by the BMP Type for this drainage basin / outfall area. \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ \\ \\ Rainfall Depth = \\ \\ \\ Post Development Runoff Coefficient = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	2.69 3652 3630 0.99	acres lbs
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36	$L_{R} = \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area \\ \\ Desired L_{M THIS BASIN} = \\ \\ F = \\ \\ Quired by the BMP Type for this drainage basin / outfall area. \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ On-site Water Quality Volume = \\ \\ Off-site area draining to BMP = \\ \\ Off-site area draining to BMP = \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ \\ Off-site area draining to BMP = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	2.69 3652 3630 0.99 3.66 0.40 33086	acres lbs lbs. inches cubic feet acres
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	$L_{R} = \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area \\ \\ Desired L_{M THIS BASIN} = \\ \\ F = \\ \\ Quired by the BMP Type for this drainage basin / outfall area. \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ On-site Water Quality Volume = \\ \\ Off-site area draining to BMP = \\ \\ Impervious fraction of off-site area = \\ \\ \\ Impervious fraction of off-site area = \\ \\ \\ \\ Impervious fraction of off-site area = \\ \\ \\ \\ \\ Impervious fraction of off-site area = \\ \\ \\ \\ \\ Impervious fraction of off-site area = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0.00	acres lbs lbs. inches cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	$L_{R} = \\ L_{R} = \\ \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area \\ \\ Desired L_{M THIS BASIN} = \\ \\ F = \\ \\ Quired by the BMP Type for this drainage basin / outfall area. \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ On-site Water Quality Volume = \\ \\ Off-site area draining to BMP = \\ \\ Off-site Impervious cover draining t$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0.00	acres lbs lbs. inches cubic feet acres
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	$L_{R} = \\ L_{R} = \\ L_{R} = \\ L_{R} = \\ \\ L_{R} = \\ \\ L_{R} = \\ \\ L_{R} = \\ \\ \\ Desired L_{M \ THIS \ BASIN} = \\ \\ F = \\ \\ \\ Quired \ by \ the \ BMP \ Type \ for \ this \ drainage \ basin / outfall \ area. \\ \\ Rainfall \ Depth = \\ \\ Post \ Development \ Runoff \ Coefficient = \\ \\ On-site \ Water \ Quality \ Volume = \\ \\ Off-site \ area \ draining \ to \ BMP = \\ \\ Impervious \ fraction \ of \ off-site \ area = \\ \\ Off-site \ Runoff \ Coefficient = \\ \\ Off-site \ Water \ Quality \ Volume = \\ \\ Output \ Part \ Quality \ Volume = \\ \\ Output \ Quality \$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0.00 0.00 0.00	acres lbs lbs. inches cubic feet acres acres
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	$L_{R} = \\ L_{R} = \\ L_{R} = \\ \\ Runoff to Treat the drainage basin / outfall area \\ \\ Desired L_{M THIS BASIN} = \\ \\ F = \\ \\ Quired by the BMP Type for this drainage basin / outfall area. \\ \\ Rainfall Depth = \\ \\ Post Development Runoff Coefficient = \\ \\ On-site Water Quality Volume = \\ \\ Off-site area draining to BMP = \\ \\ Off-site Impervious cover draining to BMP = \\ \\ Impervious fraction of off-site area = \\ \\ Off-site Runoff Coefficient =$	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00	acres lbs lbs. inches cubic feet acres acres cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	$L_R = L_R = L_R = L_R = L_R = L_R = Runoff to Treat the drainage basin / outfall area$ $Desired L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = Runoff L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = Runoff L_{M THIS BA$	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00	acres lbs lbs. inches cubic feet acres acres cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348	$L_R = L_R = L_R = L_R = L_R = L_R = Runoff to Treat the drainage basin / outfall area$ $Desired L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = Runoff L_{M THIS BASIN} = F = Runoff L_{M THIS BASIN} = Runoff L_{M THIS BA$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0.00 0.00 0.00 6.00 0.00 0.00	acres lbs lbs. inches cubic feet acres acres cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System	$L_{R} = L_{R} = L_{R$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0.00 0.00 0.00 1.00 1.00 1.00 1.0	acres lbs lbs. inches cubic feet acres acres cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348	$L_{R} = \\ L_{R} = \\ L_{R} = \\ \\ L_{R} = \\ \\ \\ L_{R} = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 NA	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348	$L_{R} = L_{R} = L_{R$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 0 0.00 0 0.00 1 0 0.000 1 0 0.0000 1 0 0.00000 1 0 0.0000 1 0 0.00000 1 0 0.00000 1 0 0.00000 1 0 0.00000 1 0 0.00000 1 0 0.00000 1 0 0.00000	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348	$\begin{array}{c} A_{P} = \\ L_{R} = \\ \hline \\ Runoff to Treat the drainage basin / outfall area \\ \hline \\ Desired L_{M This Basin} = \\ \hline \\ F = \\ \hline \\ quired by the BMP Type for this drainage basin / outfall area. \\ \hline \\ Rainfall Depth = \\ Post Development Runoff Coefficient = \\ On-site Water Quality Volume = \\ \hline \\ Off-site Impervious cover draining to BMP = \\ \hline \\ Impervious fraction of off-site area = \\ \hline \\ Off-site Runoff Coefficient = \\ \hline \\ Off-site Water Quality Volume = \\ \hline \\ Storage for Sediment = \\ \hline \\ Total Capture Volume (required water quality volume(s) x 1.20) = \\ \hline \\ Required Water Quality Volume for retention basin = \\ \hline \\ Irrigation Area Calculations \\ \hline \\ Soil infiltration/permeability rate = \\ \hline \\ Irrigation area = \\ \hline \end{array}$	2.69 3652 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 0 0.00 1 0.0	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = Quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46	$\begin{array}{c} A_{P} = \\ L_{R} = \\ \hline \\ Runoff to Treat the drainage basin / outfall area \\ \hline \\ Desired L_{M This Basin} = \\ \hline \\ F = \\ \hline \\ quired by the BMP Type for this drainage basin / outfall area. \\ \hline \\ Rainfall Depth = \\ Post Development Runoff Coefficient = \\ On-site Water Quality Volume = \\ \hline \\ Off-site Impervious cover draining to BMP = \\ \hline \\ Impervious fraction of off-site area = \\ \hline \\ Off-site Runoff Coefficient = \\ \hline \\ Off-site Water Quality Volume = \\ \hline \\ Storage for Sediment = \\ \hline \\ Total Capture Volume (required water quality volume(s) x 1.20) = \\ \hline \\ Required Water Quality Volume for retention basin = \\ \hline \\ Irrigation Area Calculations \\ \hline \\ Soil infiltration/permeability rate = \\ \hline \\ Irrigation area = \\ \hline \end{array}$	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet
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6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = Quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0	acres lbs lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = Quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = stem Required Water Quality Volume for extended detention basin =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0	inches cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = Stem Required Water Quality Volume for extended detention basin = Water Quality Volume for sedimentation basin =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 1 0	acres lbs lbs. lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet
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6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = Stem Required Water Quality Volume for extended detention basin = Mater Quality Volume for sedimentation basin = Minimum filter basin area =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 0 0.00 1 0	acres lbs lbs. lbs. inches cubic feet acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet
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6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = Quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = Stem Required Water Quality Volume for extended detention basin = Maximum Sedimentation basin area (2' Depth) = Minimum filter basin area = Maximum Sedimentation basin area (2' Depth) = Minimum sedimentation basin area (8' Depth) = Minimum Sedimentation basin area (8' Depth) = Mater Quality Volume for combined basins =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0 0.00 0 6617 39703 NA NA NA NA NA NA NA NA NA NA	acres lbs lbs. lbs. inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet square feet cubic feet
6. Calculate Capture Volume recalculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348	Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = Quired by the BMP Type for this drainage basin / outfall area. Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = Stem Required Water Quality Volume for extended detention basin = Maximum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum filter basin area (8' Depth) =	2.69 3652 3630 0.99 3.66 0.40 33086 0.00 0 0.00 0 0.00 0 0 6617 39703 NA NA NA NA NA NA NA NA	inches cubic feet square fee square fee square fee square fee

DA-1			
Residential Lots 47-62,	86-95, an		
		Unit	Area
Lots > 15K SF	0	5000	0
Lots 10K - 15K SF Lots < 10K SF	7 29	4000 3500	28000 101500
SUB-TOTALS	36	LOTS	129500
SOB-TOTALS	30	2.97	Acres
Additional Impervious	Area	2.7.	
Roadway w/ S		2.60	Acres
Proposed Westpoin	te Drive	0.93	Acres
Lif	t Station	0.10	Acres
300 SF/Lot Add. Imp		0.25	Acres
Parkland I		0.50	Acres
To	OTAL	7.35	Acres
DA-2			
Residential Lots 2-46 a	nd 96-107		
Lots > 15K SF	0	Unit 5000	Area
Lots 10K - 15K SF	18	4000	72000
Lots < 10K SF	39	3500	136500
SUB-TOTALS	57	LOTS	208500
Additional Tonormiana		4.79	Acres
Additional Impervious		2.51	Acres
Roadway w/ S Proposed Westpoin		1.11	Acres
300 SF/Lot Add. Imp		0.39	Acres
Recreations		0.25	Acres
Parkland I	Facilities	0.50	Acres
	Oak Run	0.70	Acres
T	OTAL	10.25	Acres
Lots 10K - 15K SF	7	4000	0 28000
Lots < 10K SF	15	3500	28000 52500
		3500 LOTS	28000 52500 80500
Lots < 10K SF SUB-TOTALS	15 22	3500	28000 52500
Lots < 10K SF	15 22 Area	3500 LOTS	28000 52500 80500
Lots < 10K SF SUB-TOTALS Additional Impervious	15 22 Area Sidewalk	3500 LOTS 1.85	28000 52500 80500 Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S	15 22 Area Sidewalk p. Cover	3500 LOTS 1.85	28000 52500 80500 Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious	15 22 Area Sidewalk p. Cover	3500 LOTS 1.85 1.05 0.15	28000 52500 80500 Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious	15 22 Area Sidewalk p. Cover Facilities	3500 LOTS 1.85 1.05 0.15 0.50	28000 52500 80500 Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland I	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55	28000 52500 80500 Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55	28000 52500 80500 Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Impervious To the second of the	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55	28000 52500 80500 Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Impervious Note: Unit value is base Edwards Aquifer Techn Area Uncaptured	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55	28000 52500 80500 Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Impervious To the second of the	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or	28000 52500 80500 Acres Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ 3 300 SF/Lot Add. Impervious Parkland Intervious Roadway w/ 3 Note: Unit value is base Edwards Aquifer Techn Area Uncaptured Residential Lots 53-68	15 22 Area Sidewalk p. Cover Facilities OTAL	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 of mice Manua	28000 52500 80500 Acres Acres Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Impervious Note: Unit value is base Edwards Aquifer Techn Area Uncaptured	Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or	28000 52500 80500 Acres Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ 3 300 SF/Lot Add. Impervious Parkland Intervious Roadway w/ 3 Note: Unit value is base Edwards Aquifer Techn Area Uncaptured Residential Lots 53-68 Lots > 15K SF	15 22 Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or unce Manu	28000 52500 80500 Acres Acres Acres Acres Acres Acres Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Impervious Roadway w/ S To the second of the	Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or mce Manu. Unit 5000 4000	28000 52500 80500 Acres Acres Acres Acres Acres Acres Acres 1 the al
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland Intervious To the second of the	Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or ance Manu. Unit 5000 4000 3500	28000 52500 80500 Acres Acres Acres Acres Acres Acres 16000 16000 38500
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ 3 300 SF/Lot Add. Impervious Parkland Intervious Output Value is base Edwards Aquifer Techn Area Uncaptured Residential Lots 53-68 Lots > 15K SF Lots 10K - 15K SF Lots < 10K SF SUB-TOTALS	Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or mce Manu. Unit 5000 4000 3500 LOTS 1.37	28000 52500 80500 Acres
Lots < 10K SF SUB-TOTALS Additional Impervious Roadway w/ S 300 SF/Lot Add. Impervious Parkland D T Note: Unit value is base Edwards Aquifer Techn Area Uncaptured Residential Lots 53-68 Lots > 15K SF Lots 10K - 15K SF Lots < 10K SF SUB-TOTALS 300 SF/Lot Add. Impervious	Area Sidewalk p. Cover Facilities OTAL d off of T ical Guida	3500 LOTS 1.85 1.05 0.15 0.50 3.55 able 3-2 or ance Manua Unit 5000 4000 3500 LOTS	28000 52500 80500 Acres Acres Acres Acres Acres Acres Acres 1 the all Area 5000 16000 38500 59500

Total Site Acreage 38.51 Total Existing Impervious Coverage: Proposed Impervious Coverage: 21.15 55% Impervious Percentage within DA's: Total Required Removal Load (L_M) 18,984 Total Desired Load Removed by Ponds: DA-1 6,910 DA-2 9,775 DA-3 3,630 20,315 Total Surplus Load Removal (LBS): 1,331 **Pond Sizing** Filter Area Required Filter Area Provided DA-1 Pond 3,366 5,278 18,310 DA-2 Pond 17,401 DA-3 Pond 3,309 7,366 Volume Required (CF) Volume Provided (CF) DA-1 Pond 63,830 DA-2 Pond 59,486 DA-3 Pond 39,703 50,272 173,588 164,100 Surplus Volume (CF): 9,488

SUMMARY TABLE

DA-1 WATER QUALITY POND (WEST)

Maximum sedimentation basin area (2' Depth) = 13463 square feet

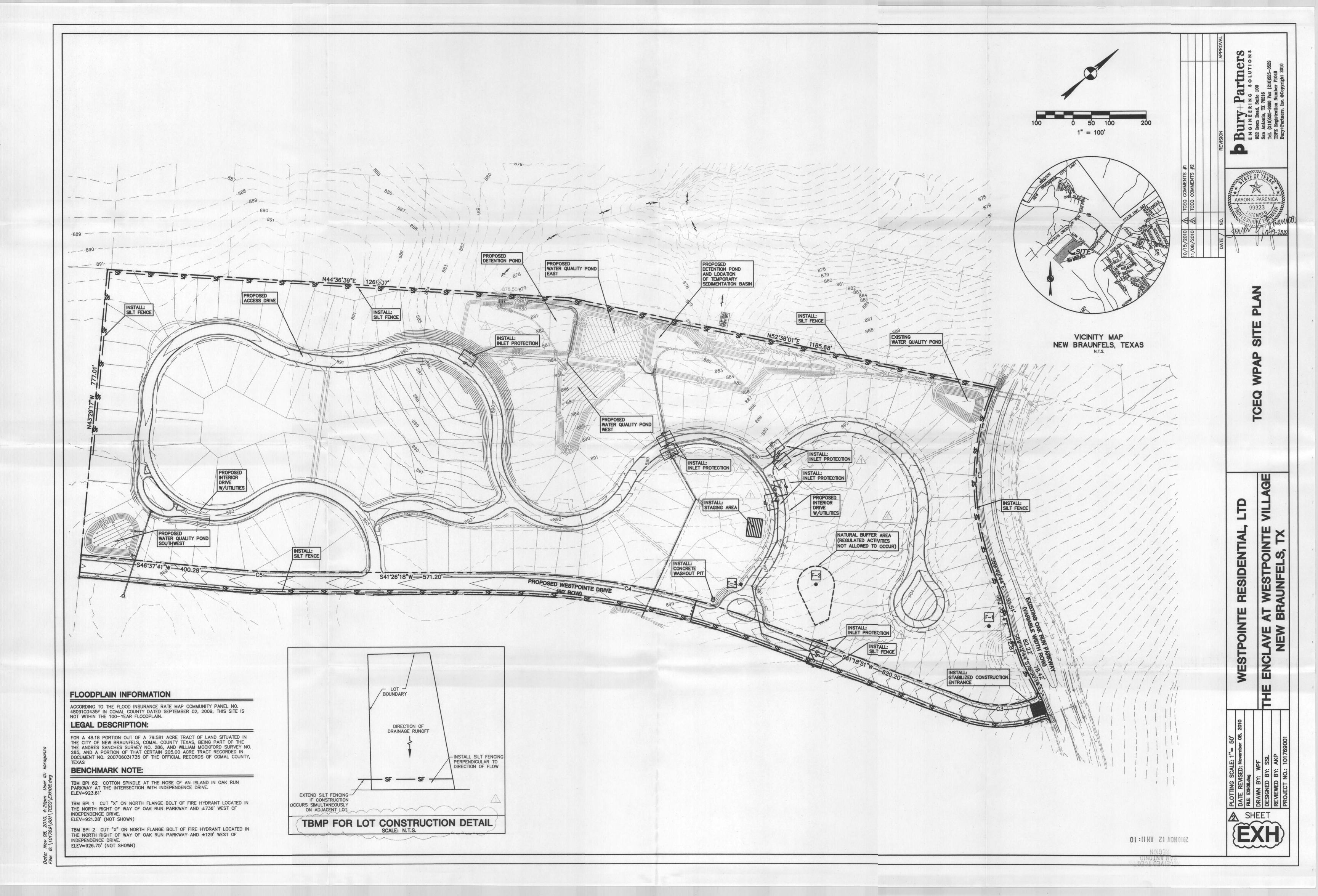
DA-2 WATER QUALITY POND (EAST)

Minimum sedimentation basin area (8' Depth) = 841 square feet

DA-3 WATER QUALITY POND (SOUTHWEST)

RESIDENTIAL

WESTPOINTE



Charlyne Fritz - Enclave at Westpointe Village - WPAP and SCS

From:

"Lin, Steve" <slin@burypartners.com>

To: Date: <cfritz@tceq.state.tx.us> 11/16/2010 3:25 PM

Subject:

Enclave at Westpointe Village - WPAP and SCS

CC:

"Parenica. Aaron" <aparenica@burypartners.com>

Attachments: C13.05 Water Quality Pond Calculations.pdf; C13.01 East Water Quality and Detention Ponds.pdf;

Enclave - Comment 2 Response.pdf

Charly,

As promised during our conversation this afternoon, attached are the two revised plan sheets stemming from the revision of the easternmost water quality pond. Also attached are the two items needed to address your SCS comments for this same project. It includes our response letter as well as an updated application fee form. Please let us know if you'll need hardcopies of any of the attached items (and quantity). We can have them sent over first thing tomorrow morning. Thanks again for all your help regarding this project.

Sincerely,

Steve L.

STEPHEN S. LIN

Engineer Associate

Bury+Partners

ENGINEERING SOLUTIONS TBPE Registration No. F1048

922 Isom Road, Suite 100 San Antonio, Texas 78216 (210) 525-9090 TEL (210) 525-0529 FAX slin@burypartners.com www.burypartners.com

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DA-1 WATER QUALITY POND (MEST)	The Common of Co	
DA-2 WATER QUALITY POND (EAST)	Control Processed Cont	CONON
DA-3 WATER QUALITY POND (BOUTHWEST)	The Common of Co	
BLIMMARY TABLE	Date	

WATER QUALITY POND CALCULATIONS

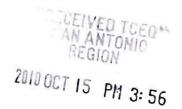
WESTPOINTE RESIDENTIAL, LTD

THE ENCLAVE AT WESTPOINTE VILLAGE NEW BRAUNFELS, TX Bury+Partners
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October 15, 2010



Project No.: R0101769-50001.61

Ms. Charly Fritz
San Antonio – Edwards Aquifer
Protection program
Texas Commission on Environmental Quality
14250 Judson Road
San Antonio, Texas 78233

Re: Edwards Aquifer, Comal County

NAME OF PROJECT: The Enclave at Westpointe Village (WPAP); Located at the SW

Corner of Oak Run Parkway and Independence Drive, New Braunfels, Texas;

PLAN TYPE: Request for the Approval to an Approved Water Pollution Abatement Plan (WPAP); 30 Texas Administrative Code (TAC) Chapter 213; San Antonio File No. 2942.00

Dear Ms. Fritz:

The following is our response to the review comments received from your office dated October 1, 2010:

General Items

1. Comment: Please provide the average area disturbed per lot. On the site plan, include a general schematic of the temporary BMPs to be implemented for individual lot construction.

Response: There are a total of 134 residential lots, comprised of ± 28.70 acres. Using Table 3-2 of the Edwards Aquifer Technical Guidance Manual, the assumed impervious coverage for all 134 lots is ± 11.26 acres. The average area disturbed per lot is 11.26/28.70 or 39% of impervious cover. Finding the average lot size by dividing 28.70 acres by 134 comes out to be 0.214 acres per lot. At 39% impervious cover per lot, this results in an average of 0.084 acres of impervious acres per lot. A general schematic of the temporary BMPs to be implemented for individual lot construction has been provided on the Site Plan.

2. Comment: To help confirm the total impervious cover for the site, please provide the assumed impervious cover per lot. If Table 3-2 from the Edwards Aquifer Technical Guidance Manual (RG-348) was used, provide information on the lot size(s) at the site and the corresponding amount of impervious cover from the table.

BURY+PARTNERS-SA, INC. 922 Isom Road, Suite 100 San Antonio, Texas 78216

> TEL (210) 525-9090 FAX (210) 525-0529



Response: Table 3-2 for the Edwards Aquifer Technical Guidance Manual (RG-348) was used to calculate the assumed impervious cover per lot. Using the calculations in response to Comment #1 above, the 134 lots are comprised of ± 28.70 acres, having an estimated 11.26 acres of impervious cover per RG-348 Table 3-2. A calculation table has been provided on the WPAP Calculation Sheet C13.05.

3. Comment: Are there any proposed parks, playgrounds, or amenity centers that will increase the impervious cover amount? A future increase in impervious cover will require a modification to this plan (if approved).

Response: Yes, each drainage area had an additional ± 0.50 acres of impervious area added to supplement facilities and amenities that may increase the overall site's impervious coverage amount. This has been annotated on the revised WPAP Calculation Sheet (C13.05).

4. Comment: The number of house lots on the SCS plan states 135 lots and the WPAP states 136. Please update either plan as necessary.

Response: There are actually 134 residential lots on this project. At this time, Lots 75 and 76 will be utilized for the Southwest Water Quality Pond.

5. Comment: Has the impervious cover associated with the lift station been accounted for in this WPAP?

Response: Yes, an area of ± 0.15 acres of impervious cover was used to account for the impervious area of the lift station in Drainage Area 1 (DA-1). See the revised WPAP Calculation Sheet.

6. Comment: On September 2, 2010, TCEQ received correspondence between Gene Majors with the Majors Law Firm PLLC and Mr. Guerra. Based upon this correspondence and the upcoming SCS review, is the proposed lift station in its final location?

Response: The proposed lift station has been relocated slightly south of the original location and will be reflected on the revised SCS and Force Main Plans to accommodate requests from the neighbor.

7. Comment: The application, specifically the Project Description, Impervious Cover Table, and BMPs for Upgrdient Storm Water, indicates the design and sizing calculations of the permanent BMPs include future land uses up-gradient of the site. This WPAP (if approved) can only grant the activities proposed in the plan. Future development and future impervious cover will be approved when a specific plan is submitted. In addition to clarifying and/or revising attachments in the application, please update the following:



- a. The Impervious Cover of the Proposed Project table states there is 31.37 acres of impervious cover but the note under the table states there is 24.20 acres of impervious cover within the 48.18 acre site. This table should reflect the impervious cover amounts for the site, not drainage areas.
- b. The permanent BMP sizing calculations appear to also be based upon drainage area. These calculations need to reflect the actual on-site drainage areas. Upgradient or offsite water either needs to be bypassed around the basins or the volume accounted for in the sizing calculations. To account for offsite or upgradient water in the TCEQ BMP sizing spreadsheet, see step #6 of the spreadsheet.

Response: The total impervious cover within the site was recalculated based on minor revisions and now totals ± 23.27 acres. The WPAP Calculation Sheet (C13.05) has been revised and now reflects the site having such amount. Furthermore, the WPAP spreadsheets have been revised as such that up-gradient or offsite water is calculated separately for the volume requirement in the Permanent Water Quality Ponds in Step #6. All revisions can be found on C13.05.

Sensitive Feature S-2

8. Comment: The natural buffer area only appears to extend approximately 100 feet from the edge of the feature. Chapter 5 of RG-348 states that the natural buffer area should extend to the boundary of the drainage area or 200 feet, whichever is less. Based upon the one foot contours it appears that additional flow from outside the indicated area will drain to the feature. Revise the drainage area so it is consistent with RG-348 or provide a statement from the project geologist indicating the natural buffer on the site plan accurately depicts the field conditions.

Response: Please see the attached letter from the project geologist in regards to the natural buffer on the site plan relative to field conditions.

9. Comment: To prevent any accidental encroachment on the natural buffer area for feature S-2, please move the staging area to another location.

Response: The Staging Area has been moved west of the original location, on the other side of the proposed drive, to create less of an encroachment hazard to the natural buffer area around Feature S-2



10. Comment: On the site plan, indicate that the area around feature S-2 is a natural buffer area where regulated activities are not allowed to occur. How will the natural buffer area and restrictions on regulated activities be conveyed to the lot/home buyers for the lots that surround the natural buffer area?

Response: The natural buffer area around Feature F-2 has been labeled on the site plan. Furthermore, the natural buffer area is annotated on the subdivision plat for this entire development, along with other "open space" areas that are not intended for regulated activities.

Site Plan and Drainage Area Map

11. Comment: Please include the TCEQ WPAP Site Plan within the bound Water Quality Pond Plans.

Response: As not all of the pond sheets were updated, we will be submitting unbound copies of only the sheets that were revised. We can collate upon delivery if necessary.

- 12. Comment: Update and/or revise the drainage area map as necessary.
 - a. Provide additional direction flow arrows.
 - b. Provide high point indicators and/or labels for the final contours for the roadways.
 - c. Drainage from Westpointe Drive into the neighborhood is unclear. At the southern entrance, portions of roadway, within DA-3, appear to drain through DA-1 in order to reach the southwest basin.
 - d. Label the areas where storm water will drain offsite without first passing through a water quality basin.
 - e. Drainage from the residential lots adjacent to the existing subdivision (western side of the site), based upon contours, appear to drain offsite and are not captured by a water quality pond.
 - f. Lots adjacent to the west water quality pond but located in DA-2 appear to drain into the west basin. See item #18 of this fax.



Response: Additional flow arrows are now shown on the drainage area map, as well as locations of swales and high points. Contours of the roadways are shown with drainage arrows showing the direction of flow and runoff. Interceptor channels will be placed behind lots to divert water to the pond inlets.

Temporary Storm Water and BMPs

13. Comment: The sizing of temporary sediment ponds is based upon the volume of runoff or 3,600 cubic feet of storage per acre drained. The statement on the Temporary Sedimentation Basin Exhibit that the total volume of the three temporary sedimentation basins exceeds the required amount (calculated from 31.37 acres) is not sufficient. Describe the amount of area disturbed in each separate drainage area then determine if the individual temporary sediment ponds are sufficient sized based upon the individual drainage areas.

Response: The sizing of the Temporary Sedimentation Basins is as follows, utilizing the disturbed acres:

Temporary Sedimentation Calculation							
Area Disturbed Volume Volume Pond Volume Acre per Acre Required Provide							
DA-1	8.06	3,600	29,016	West	63,830		
DA-2	12.57	3,600	45,252	East	59,486		
DA-3	5.05	3,600	21,060	SW	52,272		

A revised exhibit for the Temporary Sedimentation basin is included with this letter.

14. Comment: As stated in the application, the development of the site will be phased. Revise the Sequence of Major Activities (Attachment C of TCEQ—0602) to be site specific and provide the activities as they will occur in each phase.

Response: The "Sequence of Major Activities (Attachment C of TCEQ-0602) has been revised to show the activities that will occur within the phasing schedule.

15. Comment: On the site plan, provide temporary inlet protection BMPs at the inlets the water quality basins. Also provide details and specifications for all temporary BMPs used at the site.

Response: Inlet protection has been added to the inlets conveying storm water to the water quality ponds. Details of the Temporary BMPs are included for reference.



Permanent BMPs

16. Comment: Is the inlet structure leading into the west basin a pipe or a trench? The structure appears to be a trench but the Pond In-Flow detail displays a pipe leading into the basin. Are the dissipater pad and concrete blocks sufficient in preventing erosion of the sand media from a 5 foot wide trench instead of a smaller opening from a pipe?

Response: The inlet structure leading into the west basin is a channel. We have updated the detail to show both pipe and channel leading into the basin. Furthermore, the dissipater pad is sufficient to prevent erosion of the sand media at the point of discharge from the channel opening.

17. Comment: Please ensure the specifications presented on Sheet C13.04 for the Geotextile Fabric specifications meets or exceed the requirements of the revised specifications found on the RG-348 Errata sheet (attached).

Response: The detail on Sheet C13.04 for the Geotextile Fabric specification has been revised and now meets the requirements found on the RG-348 Errata sheet.

18. Comment: Based on the design requirements for the flow splitter (weir) that the structure should be capable of isolating the capture volume and bypassing additional flow around the sand filter system, revise the designs of the basins to prevent inflow from the surrounding lots or areas. All storm water to be treated by the basin must flow through the weir structure.

Response: Interceptor channels will be provided between the back of lots and the basins, which are now shown on the drainage area map. There will not be direct runoff from any of the lots into the pond(s).

19. Comment: Verify that the Goulds submersible pump will be sufficient in drawing down the water quality volume of 50,272 cubic feet within 48 hours after the rainfall event. Based upon the performance ratings for the SP035M model, the pump operates at a range between 480 GPH and 2,490 GPH. If operating at the greatest GPH amount, it will still take approximately 6 days to empty the full basin of 50,272 cubic feet or 376,060 gallon. The performance ratings were taken from the SP02/03 Product Bulletin found at http://www.goulds.com/GP-Product-ID-257.asp.

Response: Typically, the general contractor will select a supplier and meet with a pump engineer to determine the pump capacity and size so that it meets the requirements above. Inlieu-of specifying a particular pump on the plans, we have revised the note with "contractor to install submersible pump with performance rating capable of drawing down volume of pond between 24 and 48 hours" to ensure the pump will meet the required parameters.



If you have further questions or need additional information, please do not hesitate to contact me.

Sincerely,

Associate/Project Manager

Bury+Partners-SA, Inc.-F-1048

Attachments

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PROJECT DESCRIPTION

The Enclave at Westpointe Village is a ± 48.18 -acre development located near the southwest corner of the intersection of State Highway 46 and Loop 337, directly south of Oak Run Parkway. This entire development lies within the city limits of the City of New Braunfels in Comal County, Texas. The project is located in the Edwards Aquifer Recharge Zone (EARZ), and is within watershed of the Guadalupe River via the Blieders Creek tributary.

Currently, the site is undeveloped with natural vegetation and trees. There is no existing impervious coverage on the site. The proposed development will be constructed in two phases. Phase I will consist of residential subdivision units for the eastern most portion of the site fronting Oak Run Parkway. This includes the construction of an access drive into the residential subdivision, Phase I of the Sewage Collection System (SCS), and construction of the proposed Water Quality Ponds and detention basins. Phase I will total ± 24.12 -acres and increase the impervious cover by ± 9.54 -acres (40%). Phase II will consist of the remaining western half of the proposed residential subdivision, including the extension of the access drive, the addition of the western portion of the SCS, and an additional Water Quality Pond. Phase II consists of a total of ± 24.06 -acres.

Three Sand Filter Water Quality Ponds will be used as Permanent Best Management Practices (BMP's) onsite to treat stormwater generated from the development. These BMP's have been designed in accordance with TCEQ's Technical Guidance Manual to remove 80% of the increased Total Suspended Solids (TSS) for the proposed development. The proposed Water Quality Ponds have been designed to provide treatment to the entire residential subdivision as well as account for future development of the adjacent property within the entire drainage basin. Moreover, storm water will also be detained prior to being released into two existing drainage easements within the Hunters Creek subdivision. All areas not covered by the building footprint, sidewalks, or pavement will be stabilized with either sod or landscaping prior to the removal of all temporary Best Management Practices (BMPs).

The public SCS will be used to convey wastewater from the development to a Sanitary Sewer Lift station. The lift station will convey sanitary sewer through a force main into an existing waste water main operated and maintained by New Braunfels Utilities. Of the overall SCS, the gravity mains total $\pm 6,266$ linear feet of 8" SDR 26 PVC Pipe and the sanitary sewer force main totals $\pm 3,031$ linear feet. The entire SCS will comprise of two separate gravity branches, and will be constructed in accordance with the phasing plan mentioned above. The lift station along with the force main will be constructed during Phase I.

Both SCS and WPAP calculations incorporate future growth and development of the area south of the site as both the drainage and sewer sheds to the south of this development contributes to the SCS and Water Quality Ponds. This area will consist of a commercial development, with assumed impervious coverage of 95%, which will require a separate WPAP.

Based on the Geological Assessment, there are three geologic features within the project boundary. One of these features (F-2) has been deemed as sensitive. Protection of this sensitive feature will be based upon Section 5.1.2 of the TCEQ RG-348 Technical Guidance Manual, having a natural buffer extending 50 feet in all directions. The area within this buffer will be maintained in its natural state and will not have any construction activities within.

Water Pollution Abatement Plan Application

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b), Effective June 1, 1999

REGULATE	D ENTITY NAME:	THE ENCLAVE AT W	ESTPOINTE VILLA	AGE
REGULATE	ED ENTITY INFORMATI	ON		
1. The <u>✓</u> — — — — —	type of project is: Residential: # of Lots Residential: # of Livin Commercial Industrial Other:		134	
2. Tota	I site acreage (size of pr	operty): <u>48</u>	3.18 Acres	
3. Proje	ected population:		472	
4. The	amount and type of impe	ervious cover expected	after construction a	re shown below:
Impervious Project	Cover of Proposed	Sq. Ft.	Sq. Ft./Acre	Acres
Structures	/Rooftops	530,700	÷ 43,560 =	12.18
Parking		5,000	÷ 43,560 =	0.11
Other pave	ed surfaces	477,853	÷ 43,560 =	8.77
Total Impe	rvious Cover	1,013,553	÷ 43,560 =	23.27
Total Impe	rvious Cover ÷ Total Acr	reage x 100 = (31.37/53	.82) x 100	49%
5. <u>√</u>		Factors Affecting Wa ace water and groundv		
6. <u>✓</u>	Only inert materials as	defined by 30 TAC §33	0.2 will be used as fi	II material.
	PROJECTS ONLY uestions 7-12 if this app	olication is exclusively	for a road project.	
7. Type		built to county specificated to		

Street or road providing access to private driveways.

SEQUENCE OF MAJOR ACTIVITIES

The sequence of work described below will be accomplished through the timing of proposed work relating the maintenance of service (i.e. proposed utility installation as compared to the removal/abandonment of existing utilities). Below is a general sequence of events to be followed:

- 1. Obtain all required permits.
- 2. Install all Erosion Control Measures and Devices that can be installed prior to site clearing. (±24.12 acres Phase I)
- 3. Clear site for streets and ponds. $(\pm 9.10 \text{ acres Phase I})$
- 4. Install any remaining Control Measures and Devices that could not be installed prior to site clearing.
- 5. Grade site and construct temporary sedimentation pond. Install Erosion Control around catch basins. (±4.83 acres Phase I)
- 6. Set Sewage Collection System manholes and install all underground utilities and piping.
- 7. Install pavement (±4.18 acres Phase I).
- 8. Install individual residential homes (± 4.90 Acres Phase I)
- 9. Inspect and maintain all erosion control measures until all disturbed offsite and on-site areas have been hydromulched or sodded in accordance with the landscape plan and a mowable stand of grass is achieved.
- 10. Install all Erosion Control Measures and Devices that can be installed prior to site clearing. (±24.06 acres Phase II)
- 11. Clear site for streets and ponds. (±6.21 acres Phase II)
- 12. Install any remaining Control Measures and Devices that could not be installed prior to site clearing for Phase II.
- 13. Grade site and construct temporary sedimentation pond. Install Erosion Control around catch basins. (±0.50 acres Phase II)
- 14. Install pavement (±5.71 acres Phase II).

- 15. Install individual residential homes (± 6.36 Acres Phase II)
- 16. Inspect and maintain all erosion control measures until all disturbed offsite and on-site areas have been hydromulched or sodded in accordance with the landscape plan and a mowable stand of grass is achieved.

Total Site Area/Total Disturbed Area

The total area of the site is ± 48.18 acres. Excavation, grading, or other activities throughout the construction process will disturb approximately ± 26.48 acres. Post-construction impervious coverage will total ± 23.27 acres.

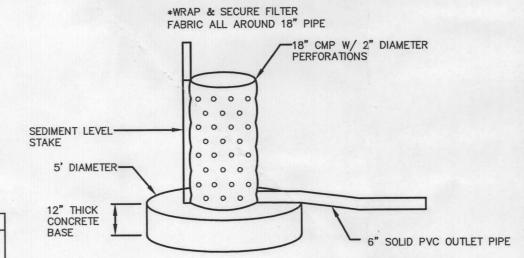
TEMPORARY SEDIMENTATION BASIN NOTES:

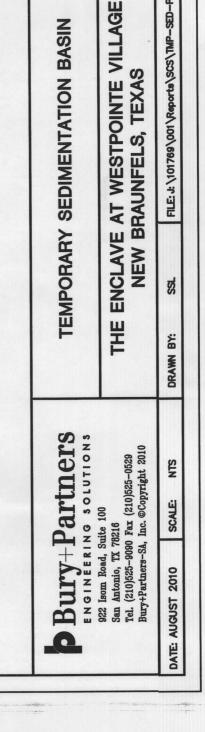
- 1. CONTRACTOR TO CONSTRUCT BASINS IN ACCORDANCE WITH CONSTRUCTION PLANS FOR PERMANENT SEDIMENTATION/FILTRATION WITH THE EXCEPTION OF THE GRAVEL DRAIN LAYER AND SAND FILTER LAYERS.
- 2. INSTALL PERMANENT STAKE TO INDICATE SEDIMENT LEVEL IN THE BASIN. STAKE SHOULD BE MARKED TO INDICATE WHEN SEDIMENT OCCUPIES 50% OF THE VOLUME OF THE BASIN.
- 3. SEDIMENT WILL BE REMOVED WHEN MORE THAN 50% OF THE BASIN CAPACITY IS EXCEDDED.
- 4. CONTRACTOR MAY USE SEED IMPREGNATED STRAW MATTING FOR SLOPE STABILIZATION. MATTING MATERIAL TO BE APPROVED BY ENGINEER.
- 5. CONTRACTOR TO SECURE PIPE TO BOTTOM OF BASIN TO PREVENT BUOYANCY DURING A RAIN EVENT. A CONCRETE ANCHOR MAY BE USED.
- 6. DISCHARGE PIPE TO BE INSTALLED SO AS TO BE IN PLACE FOR PERMANENT STRUCTURE.

TEMPORARY SEDIMENTATION BASIN CALCULATIONS:

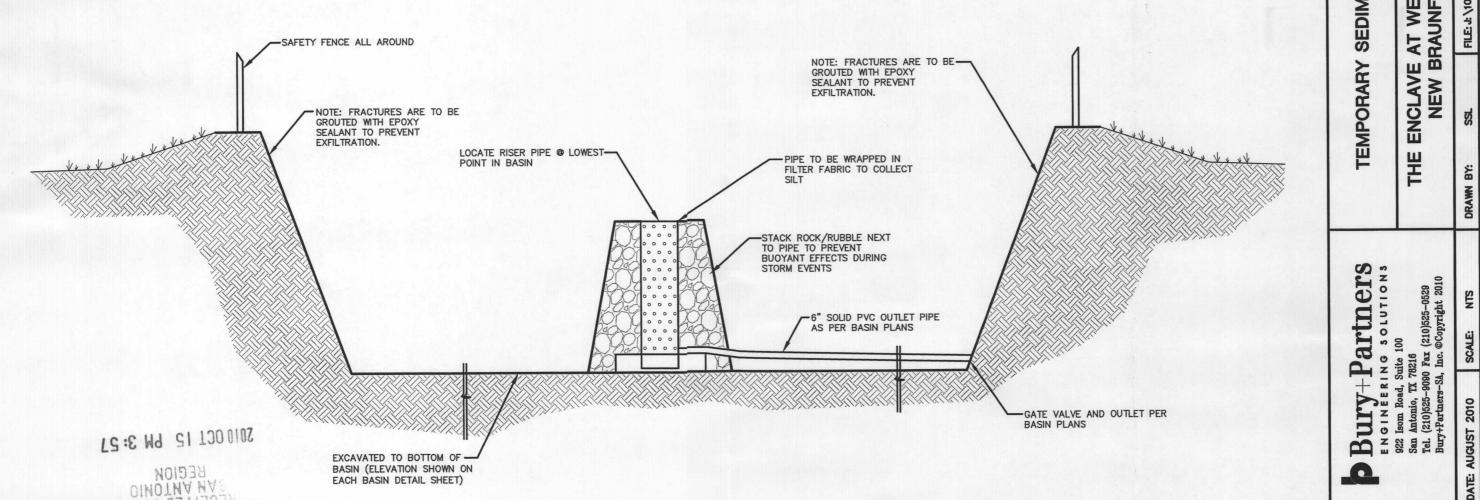
- 1. THE TEMPORARY SEDIMENTATION BASIN WILL BE LOCATED WHERE THE PROPOSED WATER QUALITY AND DETENTION POND(S) WILL BE CONSTRUCTED.
- 2. TOTAL DISTURBED AREA ONSITE IS 23.27 ACRES
- 3. 26.48 AC OF DISTURBED AREA x (3,600 CF VOLUME PER ACRE) = 95328 CF (TOTAL) OF WATER VOLUME TO BE CAPTURED IN THE TEMPORARY SEDIMENTATION BASINS
- 4. COMPARING THE VOLUMETRIC CAPACITY OF THE THREE PROPOSED WATER QUALITY PONDS AND THE DISTURBED AREA WITHIN EACH OF THE PONDS DRAINAGE BASIN, THE INDIVIDUAL VOLUME AVAILABLE FROM THE THREE TEMPORARY SEDIMENTATION BASINS IS SUFFICIENT FOR TREATMENT WITHIN EACH DRAINAGE BASIN. SEE TABLE BELOW:

	Tempo	rary Sedin	nentation Cal	culation	
Area	Disturbed Acre		Volume Required	Pond	Volume Provided
DA-1	8.06	3,600	29,016	West	63,830
DA-2	12.57	3,600	45,252	East	59,486
DA-3	5.85	3,600	21,060	SW	52,272





BMI





October 12, 2010

To Whom It May Concern:

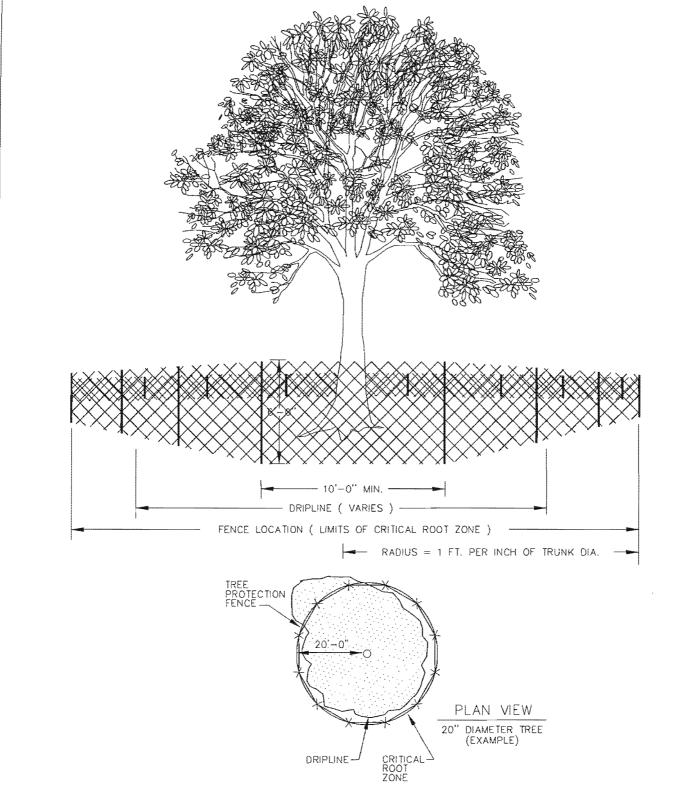
I am the project geologist for the proposed Enclave at Westpointe Village in New Braunfels, Texas. I have previously evaluated the karst feature identified as F-2. I recently conducted a review of the TCEQ WPAP site plan provided to me by Bury and Partners, which includes a 100-foot natural buffer for the feature. Following the review, I visited the site on 10/12/10 and have confirmed that the natural buffer indicated on the site plan accurately depicts the field conditions surrounding the feature. The buffer provided is more than adequate to accommodate the drainage area for the feature. If you have any questions, please feel free to contact me at (512) 694-9333 or (512) 852-3872.

Sincerely,

Stan Reece, P.G., C.A.P.M.

aci consulting

1001 Mopac Circle



TREE PROTECTION CONSTRUCTION FENCE EXHIBIT B1

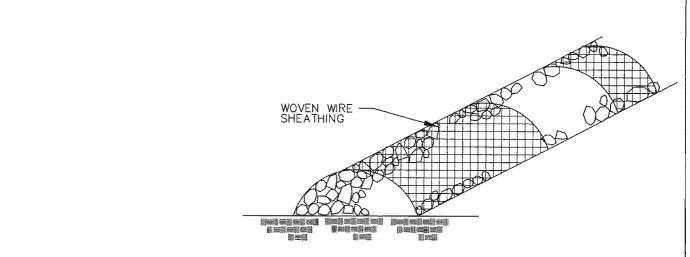
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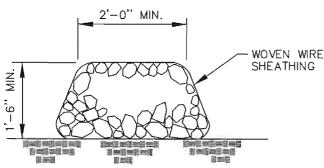
DRAWN: SL

DATE: Oct 14, 2010 SHEET: 1 OF 10 THE ENCLAVE AT WESTPOINTE VILLAGE NEW BRAUNFELS, TEXAS



ENGINEERING SOLUTIONS
922 Isom Road, Suite 100
San Antonio, TX 78216
Tel. (210)525-9990 Fax (210) 525-0529
TBPE Registration Number 71048
Bury+Partners-SA, Inc. ©Copyright 2010





NOTES:

- USE ONLY OPEN GRADED ROCK 4-8 INCH DIAMETER FOR STREAMFLOW CONDITION; USE OPEN GRADED ROCK 3-5 INCHES DIAMETER FOR OTHER CONDITIONS.
- THE ROCK BERM SHALL BE SECURED WITH A WOVEN WIRE SHEATHING HAVING MAXIMUM 1 INCH OPENINGS AND MINIMUM WIRE DIAMETER OF
- 3. THE ROCK BERM SHALL BE INSPECTED WEEKLY OR AFTER EACH RAIN, AND THE STONE AND/OR FABRIC CORE - WOVEN WIRE SHEATHING, SHALL BE REPLACED WHEN THE STRUCTURE CEASES TO FUNCTION AS INTENDED, DUE TO SILT ACCUMULATION AMONG THE ROCKS, WASHOUT, CONSTRUCTION TRAFFIC DAMAGE, ETC.
- 4. WHEN SILT REACHES A DEPTH EQUAL TO ONE-THIRD THE HEIGHT OF THE BERM OR ONE FOOT, WHICHEVER IS LESS, THE SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED SITE AND IN SUCH A MANNER AS TO NOT CREATE A SILTATION PROBLEM.
- 5. DAILY INSPECTION SHALL BE MADE ON SEVERE SERVICE ROCK BERMS; SILT SHALL BE REMOVED WHEN ACCUMULATION REACHES 6 INCHES.
- 6. WHEN THE SITE IS COMPLETELY STABILIZED, THE BERM AND ACCUMULATED SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED MANNER.

ROCK BERM

EXHIBIT B2

SCALE: NTS DRAWN: SL

DATE: Oct 14, 2010

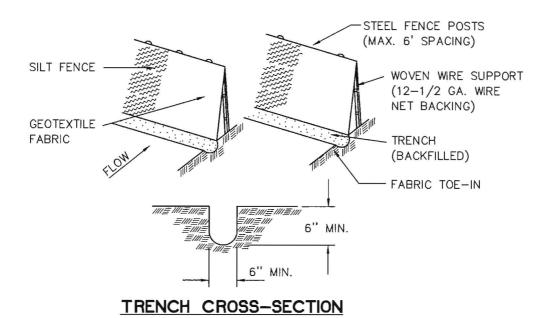
SHEET: 2 OF 10

THE ENCLAVE AT WESTPOINTE VILLAGE **NEW BRAUNFELS, TEXAS**



ENGINEERING SOLUTIONS

922 Isom Road, Suite 100 San Antonio, TX 78216 Tel. (210)525-9090 Fax (210) 525-0529 TBPE Registration Number F1048 Bury+Partners-SA, Inc. @Copyright 2010



GENERAL NOTES:

- STEEL POSTS WHICH SUPPORT THE SILT FENCE SHALL BE INSTALLED ON A SLIGHT ANGLE TOWARD THE ANTICIPATED RUNOFF SOURCE. POST MUST BE EMBEDDED A MINIMUM OF ONE FOOT.
- 2. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW. WHERE FENCE CAN NOT BE TREATED IN (e.g. pavement) WEIGHT FABRIC FLAP WITH WASHED GRAVEL ON UPHILL SIDE TO PREVENT FLOW UNDER FENCE.
- 3. THE TRENCH MUST BE A MINIMUM OF 6 INCHES DEEP AND 6 INCHES WIDE TO ALLOW FOR THE SILT FENCE FABRIC TO BE LAID IN THE GROUND AND BACKFILLED WITH COMPACTED MATERIAL.
- 4. SILT FENCE SHOULD BE SECURELY FASTENED TO EACH STEEL SUPPORT POSTS OR TO WOVEN WIRE, WHICH IS IN TURN ATTACHED TO THE STEEL FENCE POST.
- INSPECTION SHALL BE MADE WEEKLY OR AFTER EACH RAINFALL EVENT AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.
- SILT FENCE SHALL BE REMOVED WHEN THE SITE IS COMPLETELY STABILIZED SO AS NOT TO BLOCK OR IMPEDE STORM FLOW OR DRAINAGE.
- 7. ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF 6 INCHES. THE SILT SHALL BE DISPOSED OF IN AN APPROVED SITE AND IN SUCH A MANNER AS TO NOT CONTRIBUTE TO ADDITIONAL SILTATION.

SILT FENCE

EXHIBIT B3

SCALE: NTS

DRAWN: SL

DATE: Oct 14, 2010

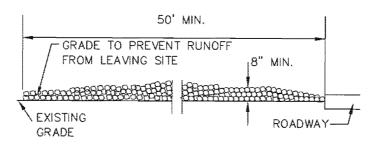
SHEET: 3 OF 10

THE ENCLAVE AT WESTPOINTE VILLAGE NEW BRAUNFELS, TEXAS

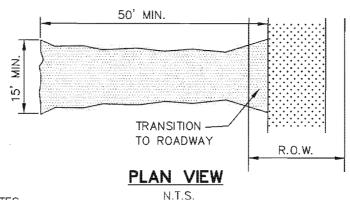
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PROFILE N.T.S.



GENERAL NOTES:

- STONE SIZE- 4 TO 6 INCH OPEN GRADED ROCK.
- LENGTH- AS EFFECTIVE, BUT NOT LESS THAN 50 FEET.
- THICKNESS- NOT LESS THAN 8 INCHES.
- WIDTH- NOT LESS THAN FULL WIDTH OF ALL POINTS OF INGRESS OR EGRESS.
- WASHING- WHEN NECESSARY, WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC ROADWAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WHICH DRAINS INTO AN APPROVED TRAP OR SEDIMENT BASIN. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH, OR WATERCOURSE USING APPROVED METHODS.
- 6. MAINTENANCE- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC ROADWAYS. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND, AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC ROADWAY MUST BE REMOVED IMMEDIATELY.
- 7. DRAINAGE- ENTRANCE MUST BE PROPERLY GRADED OR INCORPORATE A DRAINAGE SWALE TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.

STABILIZED CONSTRUCTION ENTRANCE

EXHIBIT B4

SCALE: NTS DRAWN: SL

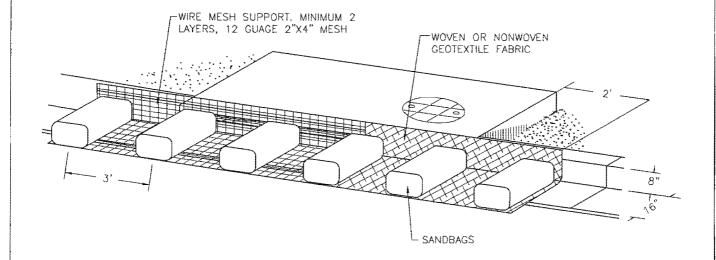
DATE: Oct 14, 2010

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THE ENCLAVE AT WESTPOINTE VILLAGE **NEW BRAUNFELS, TEXAS**



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NOTES:

- WHEN A SANDBAG IS FILLED WITH MATERIAL, THE OPEN END OF THE SANDBAG SHOULD BE STAPLED OR TIED WITH NYLON OR POLY CHORD.
- 2. INLET PROTECTION SHALL BE PLACED OVER THE MOUTH OF THE INLET WITH A $2\ \mbox{FOOT}$ OVERLAP ONEITHER SIDE.
- 3. THE FABRIC COVER AND SHALL BE A CONTINUOUS WRAPPING OF GEOTEXTILE.
- 4. THE SKIRT SHALL BE WEIGHTED WITH ONE 18"X24"X6" SANDBAG EVERY 3 FEET.
- 5. INSPECTION SHALL BE MADE WEEKLY OR AFTER EACH RAINFALL EVENT AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED BY THE CONTRACTOR.
- ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF FOUR INCHES, AND DISPOSED OF IN A MANNER WHICH WILL NOT CAUSE ADDITIONAL SILTATION.
- AFTER THE DEVELOPMENT SITE IS COMPLETELY STABILIZED, THE DIKES AND ANY REMAINING SILT SHALL BE REMOVED. SILT SHALL BE DISPOSED OF AS INDICATED IN NOTE 6 ABOVE.

CURB INLET PROTECTION BARRIER

EXHIBIT B5

DRAWN: SL

DATE: Oct 14, 2010

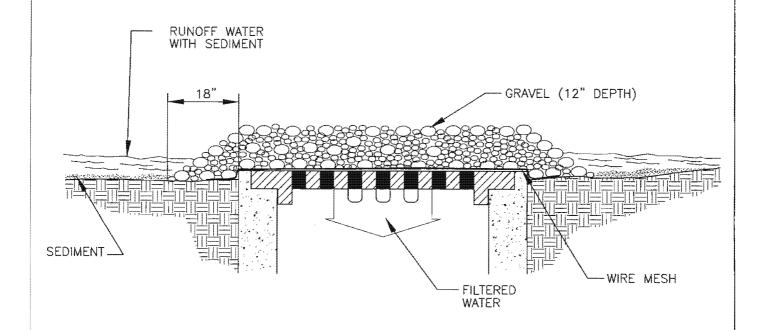
SHEET 5 OF 10

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NOTE:

- A. WIRE MESH SHALL BE LAID OVER THE DROP INLET SO THAT THE WIRE EXTENDS MINIMUM OF 1 FOOT BEYOND EACH SIDE OF THE INLET STRUCTURE. HARDWARE CLOTH OR COMPARABLE WIRE MESH WITH 1/2-INCH OPENINGS AND MINIMUM WIRE DIAMETER OF 24 GAUGE SHALL BE USED. IF MORE THAN ONE STRIP OF MESH IS NECESSARY, THE STRIPS SHALL BE OVERLAPPED.
- B. AGGREGATE SHALL BE PLACED OVER THE WIRE MESH AS INDICATED ABOVE. THE DEPTH OF STONE SHALL BE AT LEAST 12 INCHES OVER THE ENTIRE INLET OPENING. THE STONE SHALL EXTEND BEYOND THE INLET OPENING AT LEAST 18 INCHES ON ALL SIDES.
- C. IF THE STONE FILTER BECOMES CLOGGED WITH SEDIMENT SO THAT IT NO LONGER ADEQUATELY PERFORMS IT'S FUNCTION, THE STONES MUST BE PULLED AWAY FROM THE INLET, CLEANED AND REPLACED.

INLET PROTECTION/SEDIMENT FILTER

EXHIBIT B6

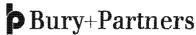
DRAWN: SL

DATE: Oct 14, 2010

SHEET 6 OF 10

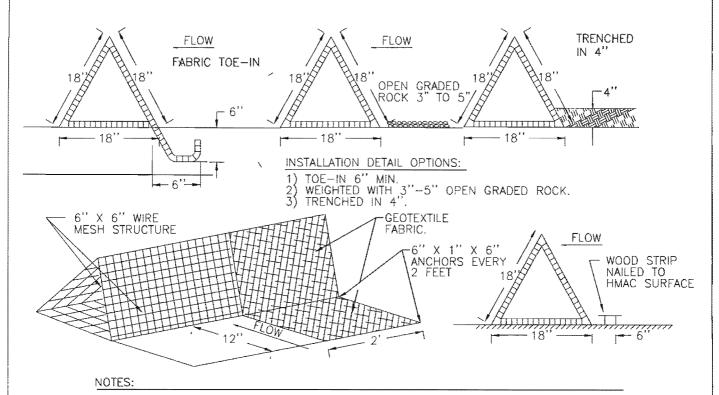
SCALE: NTS

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- DIKES SHALL BE PLACED IN A ROW WITH ENDS TIGHTLY ABUTTING THE ADJACENT DIKE.
- 2. THE FABRIC COVER AND SKIRT SHALL BE A CONTINUOUS WRAPPING OF GEOTEXTILE. THE SHIRT SHALL BE A CONTINUOUS EXTENSION OF THE FABRIC ON THE UPSTREAM FACE.
- 3. THE SKIRT SHALL BE WEIGHTED WITH, A CONTINUOUS LAYER OF 3"—5"
 OPEN GRADED ROCK, OR TOED—IN 6" WITH MECHANICALLY COMPACTED
 MATERIAL. OTHERWISE, THE ENTIRE STRUCTURE SHALL BE TRENCHED IN 4"
 INCHES
- 4. DIKES AND SKIRT SHALL BE SECURELY ANCHORED IN PLACE USING 6 INCH WIRE STAPLES ON 2 FOOT CENTERS ON BOTH EDGES AND SKIRT, OR STAKED USING 3/8 INCH DIAMETER REBAR WITH TEE ENDS.
- 5. FILTER MATERIAL SHALL BE LAPPED OVER ENDS 6 INCHES TO COVER DIKE TO DIKE JOINTS. JOINTS SHALL BE FASTENED WITH GALVANIZED SHOAT RINGS.
- 6. THE DIKE STRUCTURE SHALL BE 6 GA. 6" X 6" WIRE MESH, 18 INCHES ON A SIDE.
- INSPECTION SHALL BE MADE WEEKLY OR AFTER EACH RAINFALL EVENT AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED BY THE CONTRACTOR.
- ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF SIX INCHES, AND DISPOSED OF IN A MANNER WHICH WILL NOT CAUSE ADDITIONAL SILTATION.
- 9. AFTER THE DEVELOPMENT SITE IS COMPLETELY STABILIZED, THE DIKES AND ANY REMAINING SILT SHALL BE REMOVED. SILT SHALL BE DISPOSED OF AS INDICATED IN NOTE 8 ABOVE.

TRIANGULAR FILTER DIKE

EXHIBIT B7

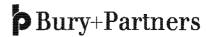
SCALE: NTS

DRAWN: SL

DATE: Oct 14, 2010

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THE ENCLAVE AT
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CONSTRUCTION SEQUENCE

- 1. OBTAIN REQUIRED PERMITS.
- 2. INSTALL ALL EROSION CONTROL MEASURES AND DEVICES THAT CAN BE INSTALLED PRIOR TO SITE CLEARING.
- 3. CLEAR SITE.
- 4. INSTALL ANY REMAINING CONTROL MEASURES AND DEVICES THAT COULD NOT BE INSTALLED PRIOR TO SITE CLEARING.
- 5. GRADE SITE.
- 6. INSTALL ALL UNDERGROUND UTILITIES. INSTALL EROSION CONTROL AROUND CATCH BASINS AND INLETS.
- 7. INSTALL PAVEMENT.
- 8. INSPECT AND MAINTAIN ALL EROSION CONTROL MEASURES UNTIL ALL DISTURBED OFFSITE & ONSITE AREAS HAVE BEEN HYDROMULCHED OR SODDED IN ACCORDANCE WITH THE LANDSCAPE PLAN AND A MOWABLE STAND OF GRASS IS ACHIEVED.

EROSION AND SEDIMENTATION CONTROL NOTES

- 1. EROSION CONTROL MEASURES, SITE WORK AND RESTORATION WORK SHALL BE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS FOR THIS PROJECT AS WELL AS THE CITY'S GENERAL REQUIREMENTS, WHICH PERTAIN TO THIS PROJECT.
- 2. ALL SLOPES SHALL BE SODDED OR SEEDED WITH APPROVED GRASS, GRASS MIXTURE OR GROUND COVER SUITABLE TO THE AREA AND SEASON IN WHICH THEY ARE APPLIED. (IN ACCORDANCE WITH LANDSCAPE PLANS)
- 3. BRUSH BERMS, HAY BALES, SEDIMENTATION BASINS AND SIMILARLY RECOGNIZED TECHNIQUES AND MATERIALS, SHALL BE EMPLOYED DURING CONSTRUCTION TO PREVENT POINT SOURCE SEDIMENTATION LOADING OF DOWNSTREAM FACILITIES. ADDITIONAL MEASURES MAY BE REQUIRED IF, THEY ARE WARRANTED.
- 4. ALL TEMPORARY EROSION CONTROL MEASURES SHALL NOT BE REMOVED UNTIL FINAL INSPECTION AND APPROVAL OF THE PROJECT BY THE CITY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN ALL TEMPORARY EROSION CONTROL STRUCTURES AND TO REMOVE EACH STRUCTURE AS APPROVED BY THE CITY.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF DUST AND DIRT RISING AND SCATTERING IN THE AIR DURING CONSTRUCTION AND SHALL PROVIDE WATER SPRINKLING OR OTHER SUITABLE METHODS OF CONTROL. THE CONTRACTOR SHALL COMPLY WITH ALL GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

TPDES REQUIREMENT NOTES

- CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING NOTICE OF INTENT (NOI) TO TCEQ FOR THE TEXAS POLLUTANT DISCHARGE ELIMINATION SYSTEM (TPDES) 48 HOURS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION ACTIVITIES, OR POSTING A CONSTRUCTION SITE NOTICE 48 HOURS PRIOR TO CONSTRUCTION ACTIVITIES.
- CONTRACTOR SHALL HAVE THIS PLAN AND THE TPDES STORM WATER POLLUTION PREVENTION PLAN ON SITE AT ALL TIMES THROUGHOUT DURATION OF PROJECT.
- ALL DISTURBED AREAS NOT ADDRESSED BY LANDSCAPE ARCHITECT SHALL BE HYDROMULCHED PER SPECIFICATION DESCRIBED IN THE GENERAL NOTES.
- CONTRACTOR SHALL PROVIDE TRIANGULAR SEDIMENT FILTER DIKE PER EXHIBIT B7 WHERE SILT FENCE IS REQUIRED BUT NOT INSTALLABLE.
- CONTRACTOR SHALL SUBMIT NOTICE OF TERMINATION (NOT) TO THE TCEQ UPON PROJECT COMPLETION AS DESCRIBED IN THE PROJECT TPDES STORM WATER POLLUTION PREVENTION PLAN. IF PROJECT IS A PHASE I PROJECT (> 5 ACRES), ELSE STABALIZE PROJECT TO WITHIN 10% OR COMPLETE CONSTRUCTION.
- CONTRACTOR TO RETAIN THE TPDES STORM WATER POLLUTION PREVENTION PLAN ALONG WITH ALL COMPLETED INSPECTION REPORTS AND PLAN MODIFICATIONS DOCUMENTATION FOR A PERIOD OF THREE (3) YEARS FROM DATE OF FINAL STABILIZATION, AS REQUIRED BY THE TCEQ.

SCALF: NTS DRAWN: SL

DATE: Oct 14, 2010

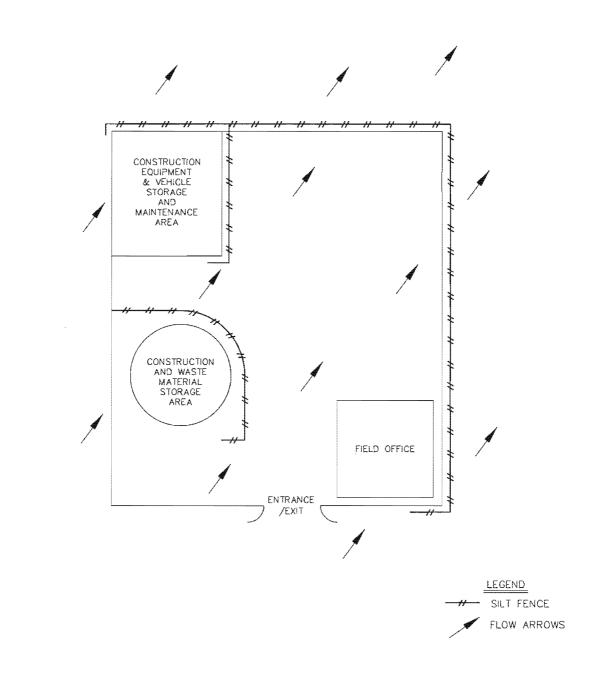
SHEET 8 OF 10

THE ENCLAVE AT WESTPOINTE VILLAGE **NEW BRAUNFELS, TEXAS**



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TYPICAL CONSTRUCTION STAGING AREA

EXHIBIT B9

SCALE: NTS
DRAWN: SL

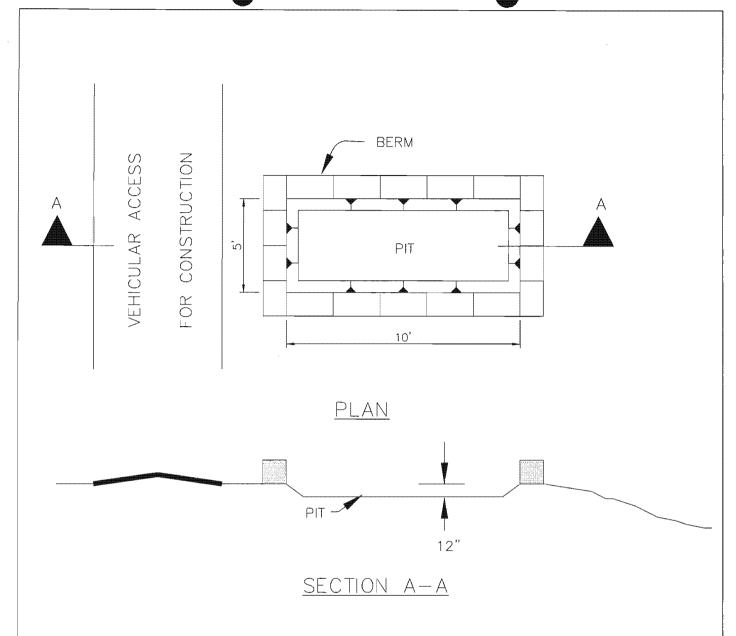
DATE: Oct 14, 2010

SHEET: 9 OF 10

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GENERAL NOTES:

- DETAIL ILLUSTRATES MINIMUM DIMENSIONS. PIT CAN BE INCREASED IN SIZE DEPENDING ON EXPECTED FREQUENCY OF USE.
- WASHOUT PIT SHALL BE LOCATED IN AN AREA EASILY ACCESSIBLE TO CONSTRUCTION TRAFFIC.
- WASHOUT PIT SHALL NOT BE LOCATED IN AREAS SUBJECT TO INUNDATION FROM STORM WATER RUNOFF.

CONCRETE TRUCK WASHOUT PIT

EXHIBIT B10

SCALE: NTS

DRAWN: SL

DATE: Oct. 14, 2010

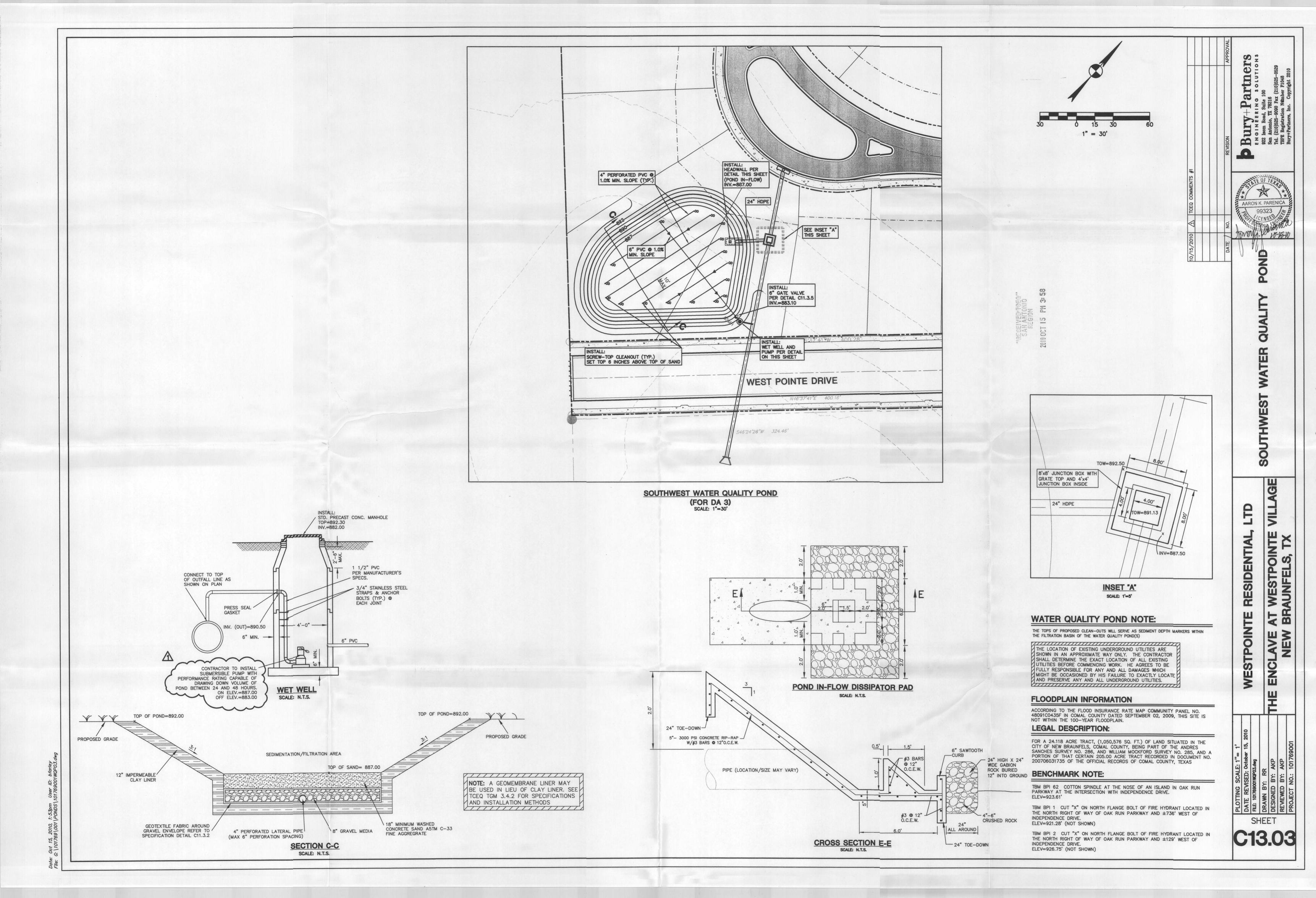
DATE: Oct 14, 2010

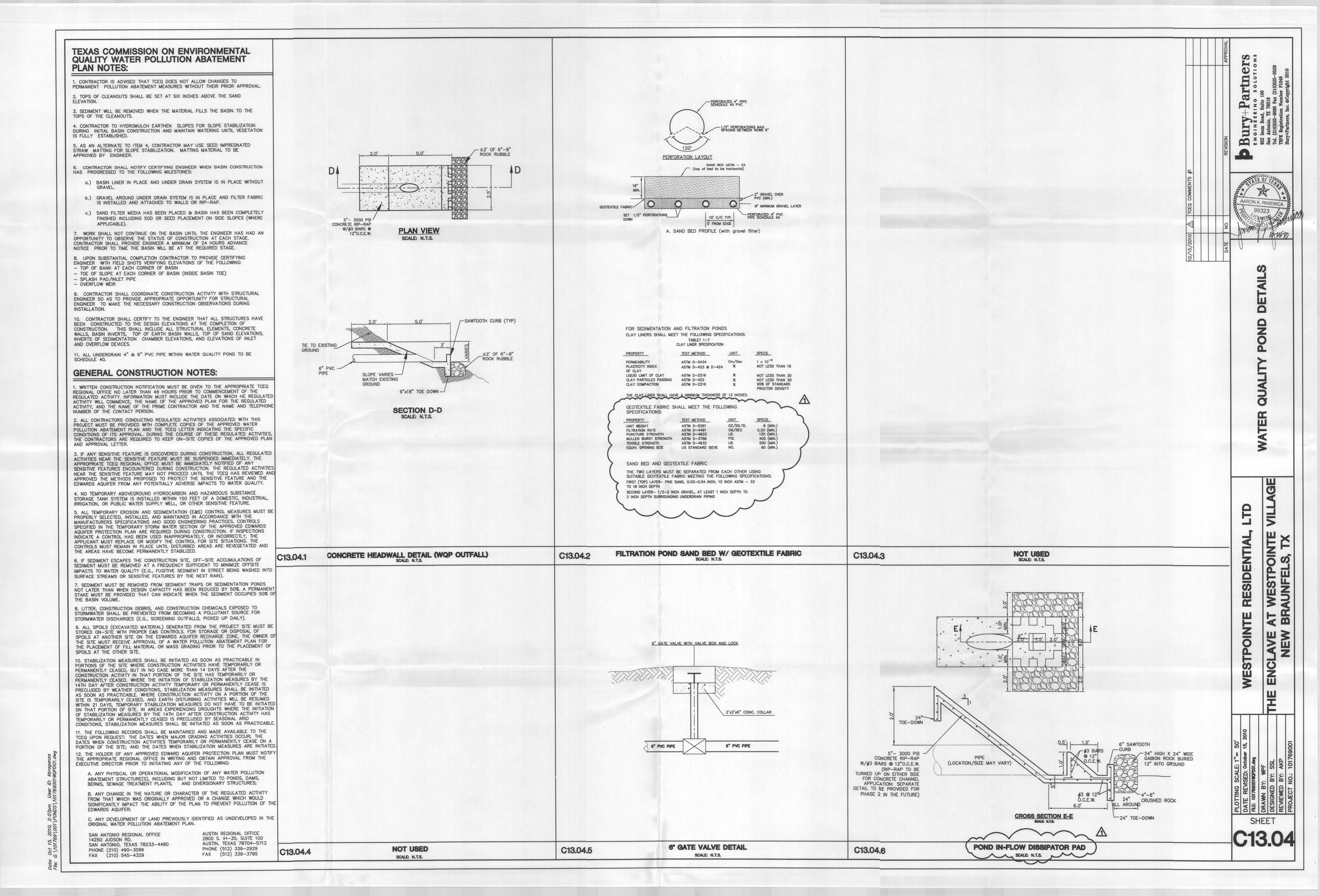
SHEET 10 OF 10

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atal Quality		
The Enclave at Westpointe Village		
For the total project:		
Page 3-29 Equation 3.3: $L_{M} = 27.2(A_{N} \times P)$		
Net increase in impervious area for the project	of increase	d load
County =	Comal	
Total project area included in plan * = Predevelopment impervious area within the limits of the plan * =	48.18 0.00	acres
otal post-development impervious area within the limits of the plan* = Total post-development impervious cover fraction * =	23.27 0.48	acres
P =	33	inches
$L_{M TOTAL PROJECT} =$	20887	lbs.
Number of drainage basins / outfalls areas leaving the plan area =	3	
ais information should be provided for each basin):		
Drainage Basin/Outfall Area No. =	2	
Total drainage basin/outfall area =	20.87	acres
Predevelopment impervious area within drainage basin/outfall area = Post-development impervious area within drainage basin/outfall area =	0.00 9.54	acres
-development impervious fraction within drainage basin/outfall area =	0.46 8563	lbs.
	0.00	.00.
	SF	abbrev
Removal efficiency =	89	percen
Removed (L _R) for this Drainage Basin by the selected BMP Type.		
RG-348 Page 3-33 Equation 3.7: $LR = (BMP \text{ efficiency}) \times P \times (A_I \times 34.6 + A_P \times 0.54)$		
Total On-Site drainage area in the BMP catchment area		
Impervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area		
TSS Load removed from this catchment area by the proposed BMP		
$A_{C} = A_{I} =$	20.87 9.54	acres acres
$A_{P} = L_{R} =$	11.33 9874	acres lbs
	8600	lbs.
F =	0.87	
the property of the last terms of the property		
uired by the BMP Type for this drainage basin / outfall area.		
	1.42	ingl
Rainfall Depth = Post Development Runoff Coefficient =	1.44 0.33	inches
Rainfall Depth =		inches cubic
Rainfall Depth = Post Development Runoff Coefficient =	0.33	
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP =	0.33 36535 0.00 0.00	cubic t
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient =	0.33 36535 0.00 0.00 0	acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area =	0.33 36535 0.00 0.00 0 0.00 0	cubic t
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient =	0.33 36535 0.00 0.00 0	acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	0.33 36535 0.00 0.00 0 0.00 0	acres acres cubic
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	0.33 36535 0.00 0.00 0 0.00 0	acres acres cubic
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842	acres acres cubic :
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations:	0.33 36535 0.00 0.00 0 0.00 0 7307 43842	cubic sacres acres cubic scubic scubi
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA	cubic acres acres cubic cubic in/hr square
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842	cubic sacres acres cubic scubic sin/hr
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA	cubic acres acres cubic cubic in/hr square
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA	cubic sacres acres cubic sin/hr square acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA	cubic sacres acres cubic sin/hr square acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA	cubic sacres acres cubic sin/hr square acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = Required Water Quality Volume for extended detention basin =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA	cubic sacres acres cubic sin/hr square acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = tem Required Water Quality Volume for extended detention basin =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA	cubic sacres acres cubic subic sin/hr square acres cubic
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = tem Required Water Quality Volume for extended detention basin = Mater Quality Volume for sedimentation basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA NA NA	cubic sacres acres cubic sin/hr square acres cubic cubic sin/hr square acres
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = tem Required Water Quality Volume for extended detention basin = Mater Quality Volume for sedimentation basin = Minimum filter basin area =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA NA NA NA	cubic sacres cubic cubic cubic cubic square
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = tem Required Water Quality Volume for extended detention basin = Mater Quality Volume for sedimentation basin = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA NA NA NA	cubic sacres cubic cubic cubic cubic square
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = Irrigation area = Mequired Water Quality Volume for extended detention basin = Maximum filtration System Water Quality Volume for sedimentation basin = Maximum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum sedimentation basin area (8' Depth) =	0.33 36535 0.00 0.00 0 0 0.00 0 7307 43842 NA NA NA NA	cubic sacres cubic cubic cubic cubic square
Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = tem Required Water Quality Volume for extended detention basin = Maximum Filtration System Water Quality Volume for sedimentation basin area = Maximum sedimentation basin area (2' Depth) = Minimum filter basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) =	0.33 36535 0.00 0.00 0 0.00 0 7307 43842 NA 0.1 NA NA NA 1.1 NA NA NA 1.2 NA 1.2 NA 1.3 NA	cubic acres acres cubic cubic cubic cubic square sq
	Page 3-29 Equation 3.3: L _M = 27.2(A _N x P) Required TSS removal resulting from the proposed development = 80% Net increase in impervious area for the project Average annual precipitation, inches Determine Required Load Removal Based on the Entire Project County = Total project area included in plan * = Predevelopment impervious area within the limits of the plan * = Total post-development impervious area within the limits of the plan * = Predevelopment impervious area within the limits of the plan * = Predevelopment impervious area within the limits of the plan * = Predevelopment impervious area within the limits of the plan * = Predevelopment impervious area leaving the plan area = Impervious for a county in the plan area = Impervious area within drainage basin/outfall area = Predevelopment impervious area within drainage basin/outfall area = Impervious area proposed in the BMP catchment area Impervious area proposed in the BMP catchment area Pervious area proposed in the BMP catchment area Pervious area remaining in the BMP catchment area Pervious area remaining in the BMP catchment area TSS Load removed from this catchment area by the proposed BMP Apple Catchment area TSS Load removed from this catchment area by the proposed BMP Apple Catchment area Desired L _{M THIS BASIN} = Catchment area Desired L _M	Page 3-29 Equation 3.3: L _M = 27.2(A _N x P) Required TSS removal resulting from the proposed development = 80% of increase Net increase in impervious area for the project Average annual precipitation, inches Determine Required Load Removal Based on the Entire Project County = County = Total project area included in plan * = Nat Nat

Project Name: The Enclave at Westpointe Village Date Prepared: 10/11/2010		
. The Required Load Reduction for the total project:		
Calculations from RG-348 Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$ Pages 3-27 to 3-30		
$L_{M TOTAL PROJECT} = Required TSS removal resulting from the proposed development = 80$ $A_{N} = Net increase in impervious area for the project$ $P = Average annual precipitation, inches$	0% of increas	ed load
Site Data: Determine Required Load Removal Based on the Entire Project		
County = Total project area included in plan * =	48.18	acres
Predevelopment impervious area within the limits of the plan * = Total post-development impervious area within the limits of the plan* =		acres acres
Total post-development impervious cover fraction * = P =		inches
$L_{M TOTAL PROJECT} =$	20887	lbs.
Number of drainage basins / outfalls areas leaving the plan area =		
2. Drainage Basin Parameters (This information should be provided for each basin):		
Drainage Basin/Outfall Area No. =	3	
		00700
Total drainage basin/outfall area = Predevelopment impervious area within drainage basin/outfall area =	0.00	acres
Post-development impervious area within drainage basin/outfall area = Post-development impervious fraction within drainage basin/outfall area =		acres
$L_{ m M\ THIS\ BASIN}=$		lbs.
3. Indicate the proposed BMP Code for this basin.		
Proposed BMP = Removal efficiency =		abbreviation percent
4. Calculate Maximum TSS Load Removed (L _R) for this Drainage Basin by the selected BMP Type.		
RG-348 Page 3-33 Equation 3.7: $LR = (BMP \text{ efficiency}) \times P \times (A_I \times 34.6 + A_P \times 0.54)$		
A_{C} = Total On-Site drainage area in the BMP catchment area		
$A_{\rm I}$ = Impervious area proposed in the BMP catchment area $A_{\rm P}$ = Pervious area remaining in the BMP catchment area		
L_R = TSS Load removed from this catchment area by the proposed BMP		
A_{C} =	7.59	acres
$A_{I} = A_{P} = A_{P}$		acres
L_{R}		lbs
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area		
Desired LM THIS PASIN	4200	lbs.
Desired $L_{M THIS BASIN} = F$		lbs.
		lbs.
\mathbf{F} =		lbs.
F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth =	= 0.88	lbs.
F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348	= 0.88 = 1.50 0.43	
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume =	= 0.88 = 1.50 0.43 = 17798	inches
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP =	= 0.88 = 1.50 0.43 = 17798 = 4.15	inches
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95	inches cubic feet acres
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78	inches cubic feet acres
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	= 0.88 = 1.50 0.43 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062	inches cubic feet acres acres cubic feet
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) =	= 0.88 = 1.50 0.43 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062	inches cubic feet acres acres
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375	inches cubic feet acres acres cubic feet cubic feet
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375	inches cubic feet acres acres cubic feet
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Required Water Quality Volume for retention basin =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375	inches cubic feet acres acres cubic feet cubic feet
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations Soil infiltration/permeability rate =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA	inches cubic feet acres acres cubic feet cubic feet
Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for retention basin = Soil infiltration/permeability rate = Irrigation area =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA	inches cubic feet acres acres cubic feet cubic feet
Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Rainfall Depth = Post Development Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = Trigation Area Calculations Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Required Water Quality Volume for extended detention basin =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA = 0.1 = NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for retention basin = Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA = 0.1 = NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Soil infiltration/permeability rate = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA = 0.1 = NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Required Water Quality Volume for retention basin = Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 P. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63	= 0.88 = 1.50 0.43 17798 = 4.15 3.94 = 0.95 = 0.78 17514 = 7062 = 42375 = NA = NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Pages 3-58 to 3-63	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA = NA = NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet in/hr square feet acres cubic feet
Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 P. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Minimum filter basin area = Minimum filter basin area =	= 0.88 = 1.50 0.43 = 17798 = 4.15 = 3.94 = 0.95 = 0.78 = 17514 = 7062 = 42375 = NA NA = NA NA = NA	inches cubic feet acres cubic feet
F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Off-site Water Quality Volume = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Pages 3-58 to 3-63	= 0.88 = 1.50 0.43 17798 = 4.15 3.94 = 0.95 = 0.78 17514 = 7062 = 42375 = NA S: = 0.1 NA NA NA NA NA = NA	inches cubic feet acres cubic feet square feet square feet square feet
Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site Area draining to BMP Impervious cover draining to BMP Impervious fraction of off-site area Off-site Area Quality Volume off-site Water Quality Volume off-site wat	= 0.88 = 1.50 0.43 17798 = 4.15 3.94 = 0.95 = 0.78 17514 = 7062 = 42375 = NA S: = 0.1 NA NA NA NA NA = NA	inches cubic feet acres acres cubic feet cubic feet cubic feet in/hr square feet acres cubic feet
6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Maximum sedimentation basin area (2' Depth) = Maximum sedimentation basin area (2' Depth) = Maximum sedimentation basin area (8' Depth) = 9B. Partial Sedimentation and Filtration System	= 0.88 = 1.50	inches cubic feet acres cubic feet square feet square feet square feet square feet square feet
6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP off-site Impervious cover draining to BMP Impervious fraction of off-site area off-site Water Quality Volume (Off-site Water Quality Volume) Storage for Sediment Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin area in Irrigation	= 0.88 = 1.50 0.43 17798 = 4.15 3.94 0.95 0.78 17514 = 7062 42375 = NA NA NA NA NA = NA = 42375 = 989 = 8899 = 2225	inches cubic feet acres cubic feet square feet square feet square feet
6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Maximum sedimentation basin area (2' Depth) = Maximum sedimentation basin area (2' Depth) = Maximum sedimentation basin area (8' Depth) = 9B. Partial Sedimentation and Filtration System	= 0.88 = 1.50 0.43 17798 = 4.15 3.94 0.95 0.78 17514 = 7062 42375 = NA NA NA NA NA = NA = 42375 = 989 = 8899 = 2225	inches cubic feet acres cubic feet square feet square feet square feet square feet square feet

DA-1			
Residential Lots 47-68, 8	6-95, an		
4577.077		Unit	Area
Lots > 15K SF	1	5000	5000
Lots 10K - 15K SF	13	4000	52000
	40 54	3500 LOTS	140000 197000
SUB-TUTALS	54	4.52	Acres
Additional Impervious A	rea	4.32	Acres
Roadway w/ Sig		2.60	Acres
Proposed Westpointe		0.93	Acres
	station	0.15	Acres
300 SF/Lot Add. Imp.	Cover	0.37	Acres
Parkland Fa	cilities	0.50	Acres
TO	TAL	9.07	Acres
DA-2 Residential Lots 1-46 and	1 06 107		
Acsidential Lots 1-40 and	1 70-10/	Unit	Area
Lots > 15K SF	1	5000	5000
Lots 10K - 15K SF	18	4000	72000
Lots < 10K SF SUB-TOTALS	39 58	3500 LOTS	136500 213500
SUD-TOTALS	36	4.90	Acres
Additional Impervious A	rea		110105
Roadway w/ Sie	dewalk	2.51	Acres
Proposed Westpointe	Drive	1.11	Acres
300 SF/Lot Add. Imp.		0.40	Acres
*Clubhouse Facility +5		0.11	Acres
Parkland Fa		0.50	Acres
10	TAL	9.54	Acres
*Clubhouse Impervious C impervious coverage alrea lot to be used	_		
DA-3			
Residential Lots 69-85, 1	08, and	131-136 Unit	Area
Lots > 15K SF	0	5000	Area 0
Lots 10K - 15K SF	6	4000	24000
Lots < 10K SF	16	3500	56000
SUB-TOTALS	22	LOTS	80000
		1.84	Acres
Additional Impervious A	rea		
Roadway w/ Si	dewalk	1.05	Acres
Proposed Westpointe	e Drive	1.12	Acres
300 SF/Lot Add. Imp.	Cover	0.15	Acres
Parkland Fa	acilities	0.50	Acres
ТО	TAL	4.66	Acres
Note: Unit value is based Edwards Aquifer Technic and 76 appropriated for V	al Guida		

Water Quality Ponds Summary Table Total Site Acreage 48.18 Total Existing Impervious Coverage: 23.27 Post-Construction Impervious Coverage: 23.27 Net Increase in Impervious Coverage: Total Required Removal Load (L_M) 20,887 Total Desired Load Removed by Ponds: 8,150 DA-2 8,600 DA-3 4,200 20,950 Total Surplus Load Removal (LBS): **Pond Sizing** Filter Area Required Filter Area Provided DA-1 Pond 17,401 DA-2 Pond 3,998 18,310 7,366 DA-3 Pond 1,653 Volume Required (CF) Volume Provided (CF) DA-1 Pond 57,054 59,486 DA-2 Pond 43,842 63,830 DA-3 Pond 42,375 50,272 173,588

SUMMARY TABLE

Surplus Volume (CF): 30,317

DA-2 WATER QUALITY POND (EAST)

DA-3 WATER QUALITY POND (SOUTHWEST)

Minimum sedimentation basin area (8' Depth) = 844 square feet DA-1 WATER QUALITY POND

Maximum sedimentation basin area (2' Depth) = 13505 square feet

cubic feet square feet Maximum sedimentation basin area (2' Depth) = 14614 Minimum sedimentation basin area (8' Depth) = 913 square feet

abbreviation

percent

acres

inches

acres

acres

cubic feet

cubic feet

cubic feet

in/hr

acres

square feet

cubic feet

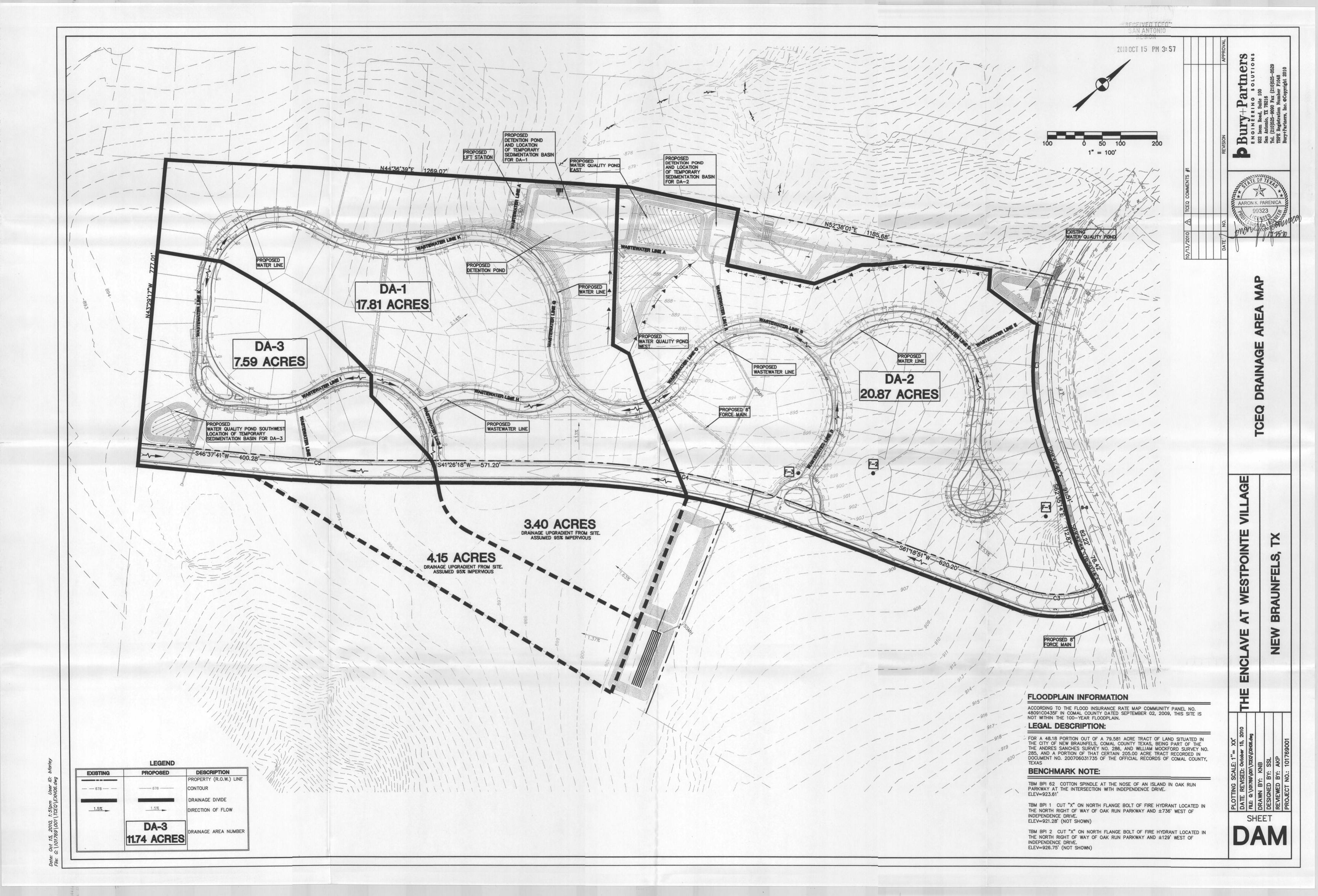
cubic feet

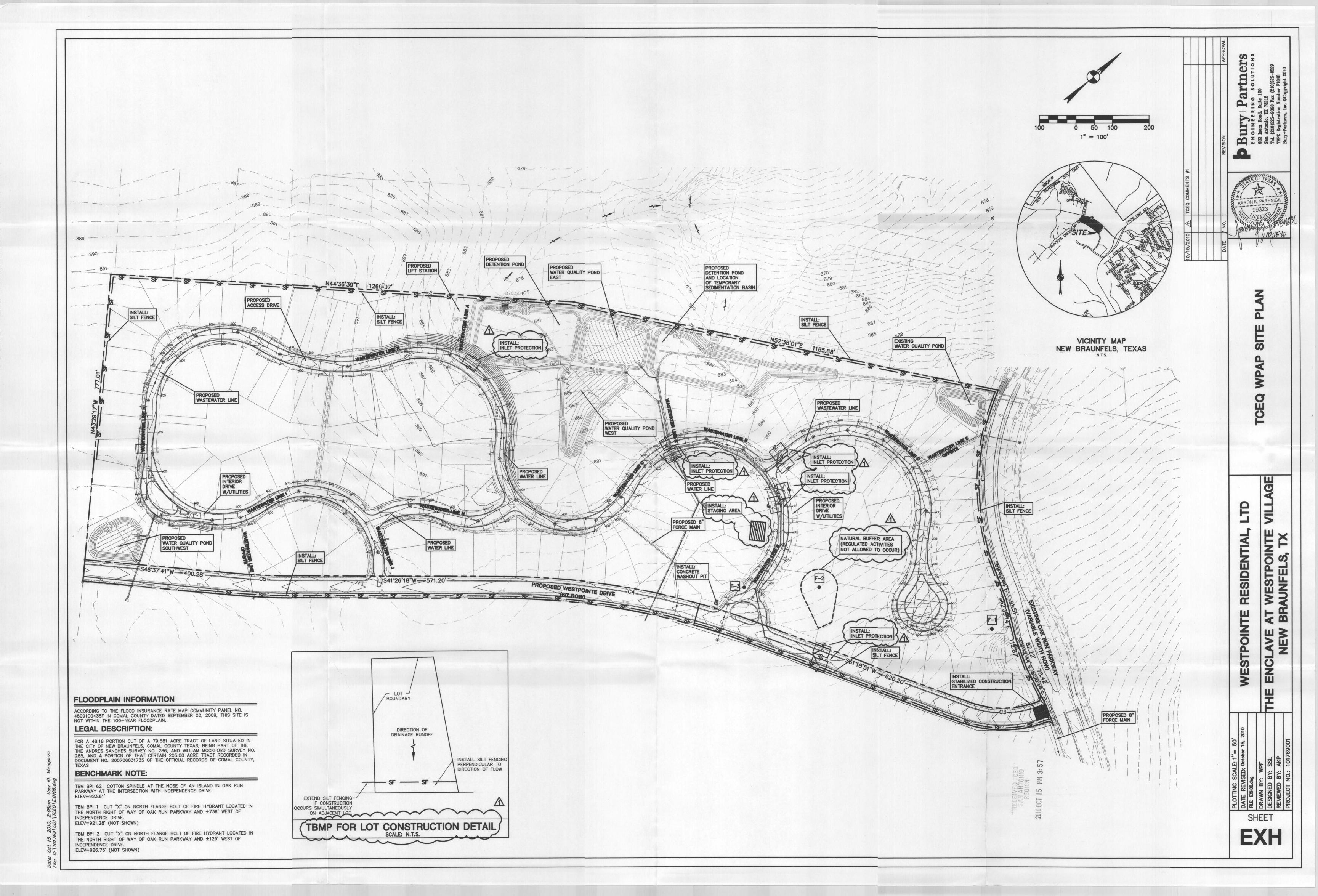
square feet

square feet

square feet

cubic feet





Bryan W. Shaw, Ph.D., Chairman
Buddy Garcia, Commissioner
Carlos Rubinstein, Commissioner
Mark R. Vickery, P.G., Executive Director



TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Protecting Texas by Reducing and Preventing Pollution

August 25, 2010

Mr. Thomas H. Hornseth, P.E. Comal County Engineer 195 David Jonas Drive New Braunfels TX 78132-3710

Re: Edwards Aquifer, Comal County

PROJECT NAME: The Enclave at Westpointe Village, located on the south side of Oak

Run Parkway, New Braunfels, Texas

PLAN TYPE: Application for Approval of a Water Pollution Abatement Plan (WPAP) 30 Texas Administration Code (TAC) Chapter 213; Edwards Aquifer Protection Program

EAPP File No.: 2942.00

Dear Mr. Hornseth:

The referenced application is being forwarded to you pursuant to the Edwards Aquifer Rules. The Texas Commission on Environmental Quality (TCEQ) is required by 30 TAC Chapter 213 to provide copies of all applications to affected incorporated cities and underground water conservation districts for their comments prior to TCEQ approval.

Please forward your comments to this office by September 24, 2010.

The Texas Commission on Environmental Quality appreciates your assistance in this matter and your compliance efforts to ensure protection of the State's environment. If you or members of your staff have any questions regarding these matters, please feel free to contact the San Antonio Region Office at (210) 490-3096.

Sincerely

Todd Jones

Water Section Work Leader San Antonio Regional Office

TJ/eg

Water Pollution Abatement Plan

For

RECEIVED

THE ENCLAVE AT AUG 3 1 2010
WESTPOINTE VILLAGE
OUNTY ENGINEER
New Braunfels, Texas

AUGUST 2010



Prepared By:

Bury + Partners-SA, Inc. 922 Isom Road, Suite 100 San Antonio, Texas 78216

Office: 210-525-9090/Fax: 210-525-0529 TBPE Registration Number F1048

I:\101769\50001\Reports\WPAP\Flysheet.doc.sg.mm



TCEQ Core Data Form

TCEQ Use Only

or detailed instructions regarding completion of this form, please read the Core Data Form Instructions or call 512-239-5175.

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	<u> </u>			N/A						
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(210) 495-8777			N	I/A					99-4217		
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42. Telephone Number	r .	43. Ext./Code		ax Numbe		T	750, 515	il Addre			
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SECTION V: A	utho	rized Signa	<u>ture</u>								
46. By my signature by and that I have signature updates to the ID num	re aut	hority to submit	this form								
(See the Core Data Fo	orm in	structions for m	ore inform	nation on	who shoul	d sign	ı this	form.)			
Company: W	estpo	inte Residenti	ial, LTD		Job '	Title:	Pa	artner			
Name(In Print):	Gu	erra, Jr.							Phone:	(210)49	95-8777
Signature: Date: 8/06/24/0					2010						

TCEQ-10400 (09/07) Page 2 of 2

GENERAL INFORMATION FORM

General Information Form

For Regulated Activities on the Edwards Aquifer Recharge and Transition Zones and Relating to 30 TAC §213.4(b) & §213.5(b)(2)(A), (B) Effective June 1, 1999

REGU	ILATED ENTITY NAM	E: THE ENCLAY	/E AT WESTPOINTE \	/ILLAGE
COUN	ITY: Comal		_ STREAM BAS	IN: Bleiders Creek
EDWA	ARDS AQUIFER:	✓ RECHARGE ZON _ TRANSITION ZOI		
PLAN	TYPE:	✓ WPAP _ SCS	AST UST	EXCEPTION MODIFICATION
CUST	OMER INFORMATIO	N		
1.	Customer (Applicant)):		
	Contact Person: Entity: Mailing Address: City, State: Telephone:	J.L. Guerra, Jr. Westpointe Reside c/o The ML&E Com Colonial Heights, V (210) 495-8777	npany, P.O. Box 212 'A	Zip: <u>23834</u> (210) 499-4217
	Agent/Representative	e (If any):		
	Contact Person: Entity: Mailing Address: City, State: Telephone:	Aaron K. Parenica, Bury+Partners 922 Isom Road, Su San Antonio, Texas (210) 525-9090	3	Zip: <u>78216</u> (210) 525-0529
2.	This project is	s inside the city limits on soutside the city limits on the city limits of the city limits on the city limits of the city limits on the city limits of the city limit	·	s, Texas
3.		e TCEQ's Regional sta		ion provides sufficient detail project and site boundaries
	Southside of Oak Ru	n Parkway, directly we	st of the Westpointe Vi	llage Shopping Center
4.		IT A - ROAD MAP. A e is attached at the en		ections to and the location of

ATTACHMENT B - USGS / EDWARDS RECHARGE ZONE MAP. A copy of the official 7 ½ minute USGS Quadrangle Map (Scale: 1" = 2000') of the Edwards

Recharge Zone is attached behind this sheet. The map(s) should clearly show:

✓

5.

	e Name(s). Recharge Zone (and Transition Zone, if applicable). In the project to the boundary of the Recharge Zone.
locate the boundaries ar manmade features noted	is provided on the project to allow TCEQ regional staff to add alignment of the regulated activities and the geologic or in the Geologic Assessment. The TCEQ must be able to or the application will be returned.

7. **ATTACHMENT C - PROJECT DESCRIPTION**. Attached at the end of this form is a detailed narrative description of the proposed project.

8.	Existing proje	ct site conditions are noted below:
		Existing commercial site
		Existing industrial site
		Existing residential site
	<u> </u>	Existing paved and/or unpaved roads
		Undeveloped (Cleared)
	√	Undeveloped (Undisturbed/Uncleared)
	- Anna Carlotta	Other:

PROHIBITED ACTIVITIES

6.

✓

- 9. <u>✓</u> I am aware that the following activities are prohibited on the **Recharge Zone** and are not proposed for this project:
 - (1) waste disposal wells regulated under 30 TAC Chapter 331 of this title (relating to Underground Injection Control);
 - (2) new feedlot/concentrated animal feeding operations, as defined in 30 TAC §213.3;
 - (3) land disposal of Class I wastes, as defined in 30 TAC §335.1;
 - (4) the use of sewage holding tanks as parts of organized collection systems; and
 - new municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41(b), (c), and (d) of this title (relating to Types of Municipal Solid Waste Facilities).
- 10. <u>N/A</u> I am aware that the following activities are prohibited on the **Transition Zone** and are not proposed for this project:
 - (1) waste disposal wells regulated under 30 TAC Chapter 331 (relating to Underground Injection Control);
 - (2) land disposal of Class I wastes, as defined in 30 TAC §335.1; and
 - (3) new municipal solid waste landfill facilities required to meet and comply with Type I standards which are defined in §330.41 (b), (c), and (d) of this title.

ADMINISTRATIVE INFORMATION

- 11. The fee for the plan(s) is based on:
 - For a Water Pollution Abatement Plan and Modifications, the total acreage of the site where regulated activities will occur.

		For an Organized Sewage Collection System Plans and Modifications, the total linear footage of all collection system lines.
		For a UST Facility Plan or an AST Facility Plan, the total number of tanks or piping systems.
	_	A request for an exception to any substantive portion of the regulations related to the protection of water quality.
	_	A request for an extension to a previously approved plan.
12.	not su submit	ation fees are due and payable at the time the application is filed. If the correct fee is bmitted, the TCEQ is not required to consider the application until the correct fee is ted. Both the fee and the Edwards Aquifer Fee Form have been sent to the ission's:
	<u>-</u>	TCEQ cashier Austin Regional Office (for projects in Hays, Travis, and Williamson Counties) San Antonio Regional Office (for projects in Bexar, Comal, Kinney, Medina, and Uvalde Counties)
13.	<u>✓</u>	Submit one (1) original and four (4) copies of the completed application to the appropriate regional office for distribution by the TCEQ to the local municipality or county, groundwater conservation districts, and the TCEQ's Central Office.
14.	<u>√</u>	No person shall commence any regulated activity until the Edwards Aquifer Protection Plan(s) for the activity has been filed with and approved by the Executive Director.
concer	ning th	my knowledge, the responses to this form accurately reflect all information requested e proposed regulated activities and methods to protect the Edwards Aquifer. This IFORMATION FORM is hereby submitted for TCEQ review. The application was
		nica, P.E. (w/ Bury+Partners) Customer/Agent
Signat	ure of C	Suptomer/Agent Date Ons on how to fill out this form or about the Edwards Aquifer protection program, please contact us at 210/490-
		ocated in the San Antonio Region or 512/339-2929 for projects located in the Austin Region.
		led to request and review their personal information that the agency gathers on its forms. They may also have any errors corrected. To review such information, contact us at 512/239-3282.

ATTACHMENT A

ROAD MAP



922 Isom Road, Suite 100
San Antonio, TX 78216
Tel. (210)525-9090 Fax (210)525-0529
TBPE Registration Number F-1046
Bury+Partners-SA, Inc. @Copyright 2010

JULY 20, 2010

DRAWN BY: SSL

WESTPOINTE VILLAGE

NEW BRAUNFELS, TEXAS

ROAD MAP

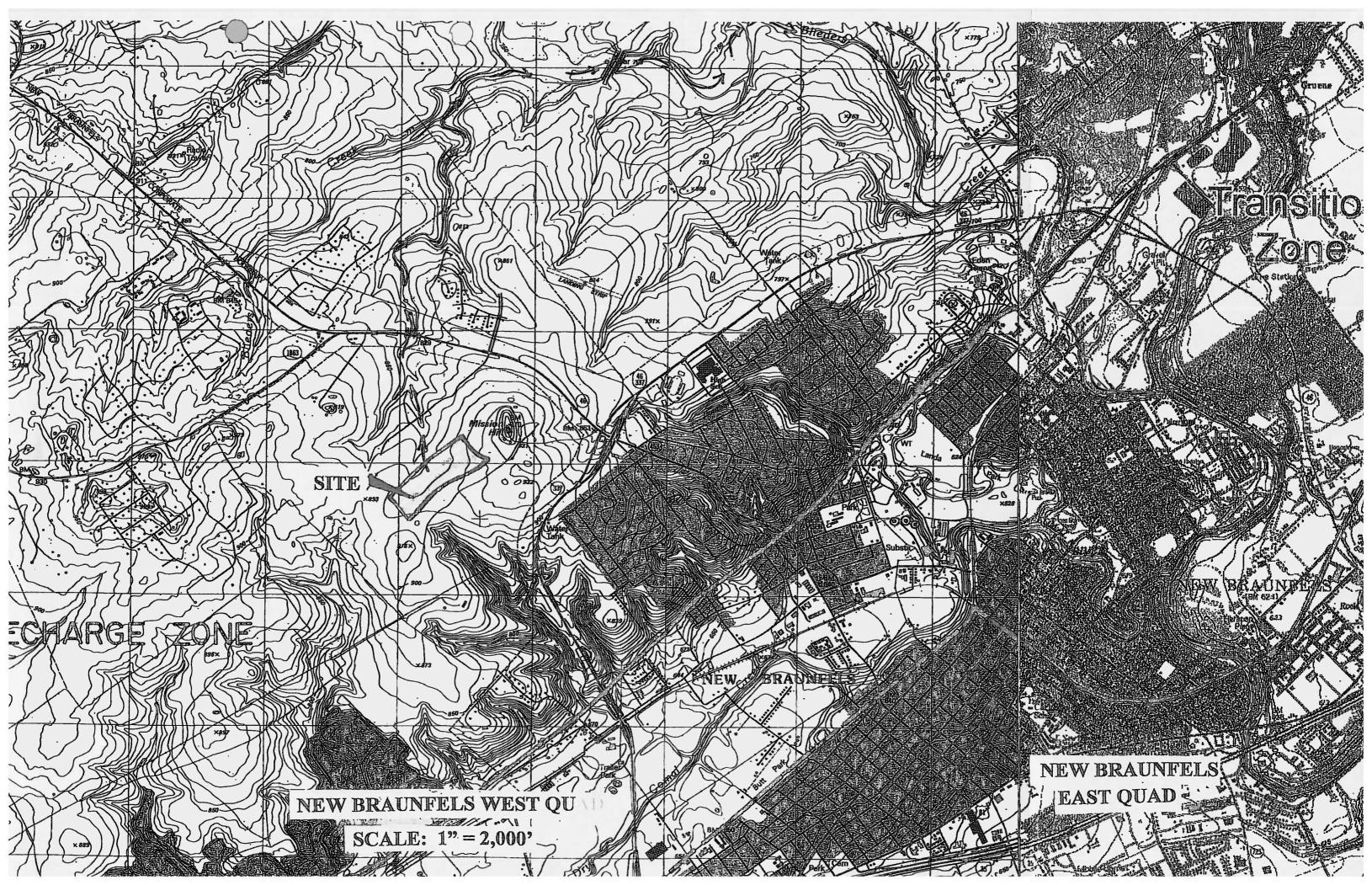
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SCALE: NTS

No.: R0101769-50001

ATTACHMENT B

USGS/EDWARDS RECHARGE ZONE MAP (Scale 1" = 2,000')



ATTACHMENT C

PROJECT DESCRIPTION

PROJECT DESCRIPTION

The Enclave at Westpointe Village is a ± 48.18 -acre development located near the southwest corner of the intersection of State Highway 46 and Loop 337, directly south of Oak Run Parkway. This entire development lies within the city limits of the City of New Braunfels in Comal County, Texas. The project is located in the Edwards Aquifer Recharge Zone (EARZ), and is within watershed of the Guadalupe River via the Blieders Creek tributary.

Currently, the site is undeveloped with natural vegetation and trees. There is no existing impervious coverage on the site. The proposed development will be constructed in two phases. Phase I will consist of residential subdivision units for the eastern most portion of the site fronting Oak Run Parkway. This includes the construction of an access drive into the residential subdivision, Phase I of the Sewage Collection System (SCS), and construction of the proposed Water Quality Ponds and detention basins. Phase I will total ± 23.93 -acres and increase the impervious cover by ± 12.33 -acres (58%). Phase II will consist of the remaining western half of the proposed residential subdivision, including the extension of the access drive, the addition of the western portion of the SCS, and an additional Water Quality Pond. Phase II consists of a total of ± 23.86 -acres.

Three Sand Filter Water Quality Ponds will be used as Permanent Best Management Practices (BMP's) onsite to treat stormwater generated from the development. These BMP's have been designed in accordance with TCEQ's Technical Guidance Manual to remove 80% of the increased Total Suspended Solids (TSS) for the proposed development. The proposed Water Quality Ponds have been designed to provide treatment to the entire residential subdivision as well as account for future development of the adjacent property within the entire drainage basin. Moreover, storm water will also be detained prior to being released into two existing drainage easements within the Hunters Creek subdivision. All areas not covered by the building footprint, sidewalks, or pavement will be stabilized with either sod or landscaping prior to the removal of all temporary Best Management Practices (BMPs).

The public SCS will be used to convey wastewater from the development to a Sanitary Sewer Lift station. The lift station will convey sanitary sewer through a force main into an existing waste water main operated and maintained by New Braunfels Utilities. Of the overall SCS, the gravity mains total $\pm 6,266$ linear feet of 8" SDR 26 PVC Pipe and the sanitary sewer force main totals $\pm 3,031$ linear feet. The entire SCS will comprise of two separate gravity branches, and will be constructed in accordance with the phasing plan mentioned above. The lift station along with the force main will be constructed during Phase I.

Both SCS and WPAP calculations incorporate future growth and development of the area south of the site as both the drainage and sewer sheds to the south of this development contributes to both the SCS and Water Quality Ponds. This area will consist of a commercial development, with assumed impervious coverage of 95%, which will require a separate WPAP.

Based on the Geological Assessment, there are three geologic features within the project boundary. One of these features (F-2) has been deemed as sensitive. Protection of this sensitive feature will be based upon Section 5.1.2 of the TCEQ RG-348 Technical Guidance Manual, having a natural buffer extending 50 feet in all directions. The area within this buffer will be maintained in its natural state and will not have any construction activities within.

BURY+PARTNERS, INC.

GEOLOGIC ASSESSMENT

Geologic Assessment

For Regulated Activities

on The Edwards Aquifer Recharge/transition Zones and Relating to 30 TAC §213.5(b)(3), Effective June 1, 1999

REGULATED ENTITY NAME:	Enclave at	Westpoint	<u>e – Comal (</u>	County, Texas	
TYPE OF PROJECT: X WPAP	AST	X SCS	UST		
LOCATION OF PROJECT: X Rech	CT: <u>X</u> Recharge Zone Transition Zone Contributing Z Transition Zone			thin the	
PROJECT INFORMATION				anomon zone	

- X Geologic or manmade features are described and evaluated using the attached GEOLOGIC ASSESSMENT TABLE.
- 2. Soil cover on the project site is summarized in the table below and uses the SCS Hydrologic Soil Groups* (*Urban Hydrology for Small Watersheds, Technical Release No. 55, Appendix A*, Soil Conservation Service, 1986). If there is more than one soil type on the project site, show each soil type on the site Geologic Map or a separate soils map.

Soil Units, Infiltration Characteristics & Thickness					
Soil Name	Group *	Thickness (feet)			
Medlin-Eckrant association (MEC), undulating	D	1-2 ft			
Krum clay (Krb) – 1 to 3 percent slopes	С	4-5 ft			
Rumple-Comfort association (RUD), undulating	D	2.5 ft			

* Soil Group Definitions
(Abbreviated)

- A. Soils having a <u>high infiltration</u> rate when thoroughly wetted.
- B. Soils having a <u>moderate infiltration</u> rate when thoroughly wetted.
- C. Soils having a <u>slow infiltration</u> rate when thoroughly wetted.
- D. Soils having a <u>very slow infiltration</u> rate when thoroughly wetted.
- 3. X A STRATIGRAPHIC COLUMN is attached at the end of this form that shows formations, members, and thicknesses. The outcropping unit should be at the top of the stratigraphic column.
- 4. X A NARRATIVE DESCRIPTION OF SITE SPECIFIC GEOLOGY is attached at the end of this form. The description must include a discussion of the potential for fluid movement to the Edwards Aquifer, stratigraphy, structure, and karst characteristics of the site.
- 5. \underline{X} Appropriate SITE GEOLOGIC MAP(S) are attached:

The Site Geologic Map must be the same scale as the applicant's Site Plan. The minimum scale is 1": 100'

Applicant's Site Plan Scale 1" = 100'Site Geologic Map Scale 1" = 100'Site Soils Map Scale (if more than 1 soil type) 1" = 100'

- 6. Method of collecting positional data:
 - X Global Positioning System (GPS) technology.
 - __ Other method(s).
- 7. X The project site is shown and labeled on the Site Location Map.

- 8. Χ Surface geologic units are shown and labeled on the Site Topographic Map.
- 9 Х Geologic or manmade features were discovered on the project site during the field investigation. They are shown and labeled on the Site Feature Map and are described in the attached Geologic Assessment Table.
 - Geologic or manmade features were not discovered on the project site during the field investigation.
- 10. The Recharge Zone boundary is shown and labeled, if appropriate.
- 11. All known wells (test holes, water, oil, unplugged, capped and/or abandoned, etc.):
 - There are water wells present within the project corridor study area and the locations are shown and labeled. (Check all of the following that apply.)
 - The wells are not in use and have been properly abandoned.
 - The well is not in use and will be properly abandoned.
 - The well are in use and complies with 16 TAC §76.
 - Х There are no wells or test holes of any kind known to exist on the project site.

ADMINISTRATIVE INFORMATION

12. One (1) original and three (3) copies of the completed assessment have been provided. Х

Date(s) Geologic Assessment was performed: September 13 and 17, and October 10, 2007 Date(s)

STAN REECE

GEOLOGY

No. 3295

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aguifer. My signature certifies that I am qualified as a geologist as defined by 30 TAC 213.

Stan Reece P.G. Print Name of Geologist

(512) 347-9000 Telephone

(512) 306-0974

August 17, 2010 Date

Signature of Geologist

Representing: __aci consultir

(Name of Company

If you have questions on how to fill out this form or about the Edwards Aquifer Protection Program, please contact us at 512/939-2929 (Austin) or 210/403-4024 (San Antonio).

Individuals are entitled to request and review their personal information that the agency gathers on its forms. They may also have any errors in their information corrected. To review such information, contact us at 512/239-3282.



GEOLOGIC ASSESSMENT FOR THE **ENCLAVE AT WESTPOINTE** WATER POLLUTION ABATEMENT PLAN (WPAP) **AND** SEWAGE COLLECTION SYSTEM (SCS)

Comal County, Texas

August 2010

Prepared for:

Westpointe Residential, Inc. 11202 Disco Drive San Antonio, Texas 78216

Prepared by:

aci consulting 1001 Mopac Circle Austin, Texas 78746

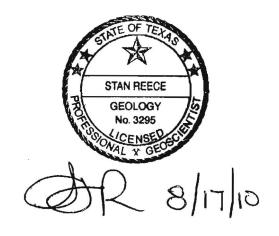




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1.0	INTRODUCTION	3
2.0	SCOPE	3
3.0	INVESTIGATION METHOD	3
4.0	PROPOSED SURVEY AREA USE	4
5.0	REGIONAL AND SITE GEOLOGY	4
6.0	KARST FEATURES IN COMAL COUNTY, TEXAS	4
7.0	SITE SOILS	5
8.0	PREVIOUS SITE INVESTIGATIONS	
9.0	DESCRIPTION OF SITE FEATURES	
10.0	SUMMARY OF FINDINGS	7
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Figure 2: Stratigraphic Column
Figure 3: Topographic Map with Formation Outcrops
Figure 4: Site Soils
Figure 5: Feature Locations



August 17, 2010

Geologic Assessment for the 54-acre Enclave at Westpointe Sewage Collection System (SCS), Comal County, Texas

1.0 INTRODUCTION

The purpose of this task is to identify "karst" features during a pedestrian survey for the property known as the Enclave at Westpointe in New Braunfels, Comal County, Texas. The approximate 54-acre Enclave at Westpointe property, hereafter referred to as the subject area, is located at the northwest corner of State Loop 337 and Highway 46 in New Braunfels, Comal County, Texas (Figure 1).

2.0 SCOPE

This report is intended to satisfy the requirements for a Geologic Assessment, which shall be included as a component of a Water Pollution Abatement Plan (WPAP) and Sewage Collection System (SCS). The scope of the report consists of a site reconnaissance and field survey and review of existing data and reports. Features identified during the field survey are ranked utilizing the Texas Commission on Environmental Quality (TCEQ) matrix for Edwards Aquifer Recharge Zone Features. The ranking of the features determines their viability as a recharge feature.

3.0 INVESTIGATION METHOD

The following investigation methods and activities were used to develop this report:

- A review of existing files and literature to determine the regional geology and known caves associated with the property;
- A review of past geological field reports, cave studies, and correspondence regarding the existing geologic features on the property;
- A site reconnaissance performed by a registered professional geologist to identify and examine caves, recharge features, and other significant geological features; and,
- Evaluation of collected field data and a ranking of features using the TCEQ Ranking Table 0585 for the Edwards Aquifer Recharge Zone.



4.0 PROPOSED SURVEY AREA USE

The site will be utilized for the construction of single family residential lots..

5.0 REGIONAL AND SITE GEOLOGY

The site lies within the Edwards aquifer recharge zone as defined by the TCEQ (TCEQ 2001). The geologic strata associated with the Edwards aquifer in Comal County include the Buda Limestone (Kbu) and the Del Rio formation overlying the Edwards Limestone Group. The dominant structural trend of known faults in the area is to the northeast on a bearing of approximately 40 to 50 degrees to the northeast (USGS, New Braunfels West Quadrangle, 1993).

Surface geology of the site include outcrops of the Edwards Limestone Formation (Ked) and Del Rio Clay (Kdr). Outcrops of the Edwards Limestone on the site occur as light-gray to gray, thick bedded limestone. Some outcrops are dolomitic in nature. Outcrops of Del Rio clay on the property appear as blocky medium-gray to light gray silty clay. Figure 2 depicts the stratigraphic column for the site. A topographic map with formation outcrops is included as Figure 3.

6.0 KARST FEATURES IN COMAL COUNTY, TEXASERROR! BOOKMARK NOT DEFINED.

In limestone terrains, karst is expressed by erratically developed cavernous porosity and the manifestations of sinkholes, voids, and erratic surface drainage. Karst landscapes are typical of the Edwards Limestone, occurring across a vast region of Central Texas west of the Balcones Escarpment, and these processes are critical to understanding the Edwards Aquifer within its various segments. The features produced by karst processes (voids, holes, and solution layers) eventually provide conduits for surface water runoff and "point recharge" for the Edwards aquifer. The identification and protection of these features in established recharge areas is critical to maintaining groundwater quality and species habitat. The United States Fish and Wildlife Service (USFWS) and the TCEQ require protective strategies within these areas to ensure recharge and endangered species habitat protection prior to, during, and upon completion of construction activities. The subject area is located in Comal County which is not within an area where endangered karst invertebrates exist or may be known to exist.



7.0 SITE SOILS

The description of the site soils are derived from two sources:

- Utilization of the "Soil Survey of Comal County, Texas," January, 1984, compiled by the United States Department of Agriculture (USDA) Natural Resource Conservation Service; and,
- Field observations made during the site reconnaissance.

Three soil units are identified within the subject area:

Krum clay (Krb) – 1 to 3 percent slopes – These gently sloping soils occur on stream terraces and valley hills. Typically, the surface layer consists of dark gray clay about 16 inches thick with subsoil, to a depth of 58 inches, consisting of grayish, brown clay. This soil is typically well-drained with moderate permeability.

Medlin-Eckrant association, undulating (MEC) – This association consists of very shallow and deep soils on upland areas in the Edwards Plateau area. The typical surface layer of Medlin consists of nine inches of grayish, brown clay. The subsoil is olive clay to a depth of approximately 24 inches, and mottled pale olive and pale yellow clay to a depth of 38 inches. The Medlin soil is well-drained with rapid surface runoff and slow permeability.

The Eckrant soil consists of a surface layer of dark brown extremely stony clay approximately 17 inches thick with underlying material consisting of fractured limestone bedrock. The Eckrant soil is well drained with rapid surface runoff and moderately slow permeability.

Rumple-Comfort association (RUD), undulating – This association consists of shallow and moderately deep upland soils in the Edwards Plateau area. Rumple soils make up approximately 60 percent of the association, Comfort soils make up 20 percent, and other soils, mainly Tarpley soils, make up 20 percent. The typical surface layer of the Rumple soil consists of dark reddish-brown cherty clay loam about 10 inches thick. The subsoil to a depth of 28 inches is dark reddish-brown extremely stony clay.

The surface layer of the Comfort soil is dark brown, extremely stony clay to about 7 inches. The subsoil to a depth of 12 inches is dark, reddish-brown, mildly alkaline, extremely stony clay. The underlying material is indurated non-calcareous fractured limestone throughout. All soils in this association are well-drained with moderate surface runoff.



A site soils map is included as Figure 4.

8.0 PREVIOUS SITE INVESTIGATIONS

There are no known previous site investigations conducted for this property according to information received from the property developer.

9.0 DESCRIPTION OF SITE FEATURES

All features listed below were identified and assessed by aci personnel during site visits conducted on September 13 and 17, and October 10, 2007. A total of three geologic features were identified within the development boundaries during the reconnaissance for this geologic assessment. A feature location map is included as Figure 5. All feature descriptions are identified as follows:

Feature 1

GPS: N 29.71298 W -98.16708

This feature is a sinkhole with a length, width and vertical depth of 5 feet, 4 feet, and 1.5 feet, respectively. Infill material consists of cobbles, loose soil, leaf litter, and other organic material. The feature is located on a hillside, and the drainage area appears to be less than 1.6 acres. Relative infiltration rate of this feature is low (17 points). The TCEQ Geologic Assessment sensitivity rating is 37.

Recommendations: No further activities are recommended for this feature.

Feature 2

GPS: N 29.71223 W -98.16835

This feature is a series of six solution-enlarged cavities, the largest of which has a length, width and vertical depth of 2 feet, 1 foot, and greater than 4 feet, respectively. Infill material consists of cobbles, breakdown, sand, and gravel. Drainage area appears to be less than 1.6 acres. Relative infiltration rate of this feature is intermediate (30 points). The TCEQ Geologic Assessment sensitivity rating is 50.

Recommendations: A minimum setback of 50-feet corresponding to the associated drainage area is recommended for this feature.

Feature 3

GPS: N 29.71187 W -98.16875

This feature is a natural bedrock feature with a length, width and vertical depth of 20 feet, 5 feet, and 1 foot, respectively. The feature is located on a hillside, and the drainage area



appears to be less than 1.6 acres. Relative infiltration rate of this feature is low (15 points). The TCEQ Geologic Assessment sensitivity rating is 30.

Recommendations: No further activities are recommended for this feature.

10.0 SUMMARY OF FINDINGS

A total of three geologic or manmade features identified within the subject area. Only one of the features was rated as sensitive under TCEQ guidelines.

11.0 RECOMMENDATIONS

Recommendations for each feature are included below the individual feature descriptions.



12.0 REFERENCES

- United States Geological Survey (USGS), New Braunfels West Quadrangle (1993), Bureau of Economic Geology, The University of Texas at Austin.
- Soil Conservation Service. 1984. Soil Survey of Comal County, Texas. United States Department of Agriculture. Texas Agriculture Experiment Station.
- (TCEQ) Texas Commission on Environmental Quality. 2001. "Edwards Aquifer Protection Program, Chapter 213 Rules Recharge Zone, Transition Zone, Contributing Zone, and Contributing Zone within the Transition Zone." Map. Digital data. November 28, 2001. Austin, Texas.

GEOLO	GIC ASSESSME	NT TABLE					PR	OJE	CT NA	ME		Enclav	e at V	/estpointe	WPA	o and	d SC	S		
		FEATURE CHARACTERISTICS						l I				EVALUATION			SICAL	SETTING				
1A	16 *	1C*	2A	28	3	4		5	5A	6	7 8A 86		9 10		11		12			
FEATURE ID	LATITUDE	LONGITUDE	FEATURE TYPE	POINTS	FORMATION	DIMENSIONS (FEET)				DENSITY (NO/FY)	APERTURE (FEET)	INFILL	RELATIVE INFILTRATION RATE	TOTAL SENSITIVITY		TY GATCHMENT AREA (ACRES)		TOPOGRAPHY		
						Х	Υ	z		10						~40	<u>>40</u>	⊀1,6	≥1.6	
F-1	29.71298	-98.16708	SH	20	Kdr	5	4	1.5					C,O	17	37	Х		Х		Hillside
F-2	29.71223	-98.16835	SC	20	Kdr	2	1	4+				2	С	30	50		X	X		Flat
F-3	29.71187	-98.16875	0	5	Kdr	20	5	1	NE - 30	10			N	15	30	Х		Х		Hillside
															<u> </u>					
								T												
								T	***************************************						1			T		

2A TY	PE TYPE	2B POINTS
С	Cave	30
sc	Solution cavity	20
SF	Solution-enlarged fracture(s)	20
F	Fault	20
0	Other natural bedrock features	5
MB	Manmade feature in bedrock	30
SW	Swallow hole	30
SH	Sinkhole	20
CD	Non-karst closed depression	5
z	Zone, clustered or aligned features	30

	8A INFILLING
4	None, exposed bedrock
	Coarse - cobbles, breakdown, sand, gravel
)	Loose or soft mud or soil, organics, leaves, sticks, dark colors
=	Fines, compacted clay-rich sediment, soil profile, gray or red colors
1	Vegetation. Give details in narrative description
S	Flowstone, cements, cave deposits
<	Other materials

12 TOPOGRAPHY
Cliff, Hilltop, Hillside, Drainage, Floodplain, Streambed

I have read, I understood, and I have followed the Texas Commission on Environmental Quality's Instructions to Geologists. The information presented here compiles with a recubient and the representation of the conditions observed in the field.

My signature certifies that ham qualified as a geologist as defined by 30 TAC Chapter 213.

STAN REECE GEOLOGY No. 3295 Date 8/17/2010

Sheet __1__ of ___1__

TCEQ-0585-Table (Rev. 10-01-04)

Enclave at Westpointe Figure 5. Feature Locations



Figure 2 Stratigraphic Column Enclave at Westpointe

System	Group or Formation	Thickness	Description	
Lower Cretaceous Del Rio Clay (Kdr)		Unknown	Dark gray to olive brown, calcareous clay, some pyretic.	
Edwards Limestone Lower Cretaceous (Ked)		Unknown	Mostly hard and dense, thin bedded, dark gray, fine to medium grained limestone, some dolomitic. Tree cover is sparse in western portion of formation.	

This map is intended for planning purposes only. Base mapping compiled from best available information. All map data should be considered preliminary and all boundaries and designations are subject to confirmation. This map is conceptual in nature and does not Figure 3. Enclave at Westpointe
Topography with Formation Outcrops Kbu



WATER POLLUTION ABATEMENT PLAN APPLICATION

Water Pollution Abatement Plan Application

for Regulated Activities
on the Edwards Aquifer Recharge Zone
and Relating to 30 TAC §213.5(b), Effective June 1, 1999

REGULATED E	NTITY NAME:	THE ENCLAVE	<u>AT WESTPOINTE VII</u>	_LAGE
REGULATED E	NTITY INFORMATI	ON		
✓ F — F — C	e of project is: Residential: # of Lots Residential: # of Livir Commercial Industrial		<u>136</u>	*Note: Calculations for proposed WQPs
2. Total site	Total site acreage (size of property):			are based on Total Drainage Area of 53.82 Ac to provide treatment of future commercial tract to the south of project site.
3. Projecte	B. Projected population:			
4. The amo	ount and type of imp	ervious cover expe	ected after constructio	n are shown below:
Impervious C Project	over of Proposed	Sq. Ft.	Sq. Ft./Acre	Acres
Structures/Roo	oftops	776,239	÷ 43,560 =	17.82
Parking		208,216	÷ 43,560 =	4.78
Other paved s	Other paved surfaces		÷ 43,560 =	8.77
Total Impervious Cover		1,366,477	÷ 43,560 =	31.37
Total Impervious Cover ÷ Total Acreage x 100			.37/53.82) x 100	58% *
(24.20 ac/48.18ac) 5.	x 100 = 50% Impervious	Cover. Factors Affecting	g Water Quality. A	description of any factors provided at the end of this
6. <u>✓</u> (Only inert materials as	s defined by 30 TAG	C §330.2 will be used a	s fill material.
	OJECTS ONLY tions 7-12 if this app	olication is exclus	ively for a road projec	et.
(XDOT road project. County road or roads	oads to be dedica	ated to a municipality.	

8.		of pavement or road surface to be used: Concrete Asphaltic concrete pavement Other:		
9.		of Right of Way (R.O.W.): feet. of R.O.W.: feet. = Ft² ÷ 43,560 Ft²/Acre = acres.		
10.	Length Width L x W	of pavement area: feet. of pavement area: feet. = Ft² ÷ 43,560 Ft²/Acre = acres. nent area acres ÷ R.O.W. area acres x 100 =% impervious cover.		
11.	_	A rest stop will be included in this project. A rest stop will not be included in this project.		
12.	_	Maintenance and repair of existing roadways that do not require approval from the TCEC Executive Director. Modifications to existing roadways such as widening roads/adding shoulders totaling more than one-half (1/2) the width of one (1) existing lane require prior approval from the TCEQ.		
STOR	MWATE	ER TO BE GENERATED BY THE PROPOSED PROJECT		
13.	<u> </u>	ATTACHMENT B - Volume and Character of Stormwater. A description of the volume and character (quality) of the stormwater runoff which is expected to occu from the proposed project is provided at the end of this form. The estimates o stormwater runoff quality and quantity should be based on area and type of impervious cover. Include the runoff coefficient of the site for both pre-construction and post construction conditions.		
WAST	EWATE	ER TO BE GENERATED BY THE PROPOSED PROJECT		
14.	The ch	naracter and volume of wastewater is shown below: 100% Domestic 491,702 gallons/day		
		TOTAL_491,702_gallons/day		
15.	Waste <u>N/A</u>	water will be disposed of by: On-Site Sewage Facility (OSSF/Septic Tank): N/A ATTACHMENT C - Suitability Letter from Authorized Agent. An on-site sewage facility will be used to treat and dispose of the wastewater. The appropriate licensing authority's (authorized agent) written approval is provided at the end of this form. It states that the land is suitable for the use of an on site sewage facility or identifies areas that are not suitable. Each lot in this project/development is at least one (1) acre (43,560 square feet in size. The system will be designed by a licensed professional engineer or registered sanitarian and installed by a licensed installer in compliance with 30 TAC Chapter 285.		
	✓	Sewage Collection System (Sewer Lines): Private service laterals from the wastewater generating facilities will be		

		<u>✓</u>	connected to an existing SCS. Private service laterals from the wastewater generating facilities will be connected to a proposed SCS. The SCS was previously submitted on The SCS was submitted with this application. The SCS will be submitted at a later date. The owner is aware that the SCS may not be installed prior to Executive Director approval.
			ewage collection system will convey the wastewater to the <u>Gruene Wastewater</u> nent Plant. The treatment facility is: <u>vertical existing.</u> <u>proposed.</u>
16.	\checkmark	All priv	rate service laterals will be inspected as required in 30 TAC §213.5.
SITE	PLAN R	EQUIR	EMENTS
tems	17 thro	ough 27	must be included on the Site Plan.
17.	The S	ite Plan	must have a minimum scale of 1" = 400'. Site Plan Scale: 1" = 100 '.
18.	100-y€ <u>✓</u>	Some floodpl	Iplain boundaries part(s) of the project site is located within the 100-year floodplain. The ain is shown and labeled. t of the project site is located within the 100-year floodplain.
	materi <u>Federa</u>	al) sour al Emer	floodplain boundaries are based on the following specific (including date of ces(s): gency Management Agency (FEMA) Flood Insurance Rate Map, Number Effective Date September 25, 2009
19.	<u>✓</u>	approposition appropries. The la	ayout of the development is shown with existing and finished contours at priate, but not greater than ten-foot contour intervals. Show lots, recreations, buildings, roads, etc. Yout of the development is shown with existing contours. Finished topographic rs will not differ from the existing topographic configuration and are not shown.
20.	All kno <u>N/A</u>	There	s (oil, water, unplugged, capped and/or abandoned, test holes, etc.): are(#) wells present on the project site and the locations are shown and d. (Check all of the following that apply) The wells are not in use and have been properly abandoned. The wells are not in use and will be properly abandoned. The wells are in use and comply with 16 TAC §76. There are no wells or test holes of any kind known to exist on the project site.
21.	Geolog ✓ — N/A	All ser shown No se Assess ATTAG except	anmade features which are on the site: asitive geologic or manmade features identified in the Geologic Assessment are and labeled. ansitive geologic or manmade features were identified in the Geologic ament. CHMENT D - Exception to the Required Geologic Assessment. An ion to the Geologic Assessment requirement is requested and explained at the this form.

- 23. ✓ Areas of soil disturbance and areas which will not be disturbed.
- 24. Locations of major structural and nonstructural controls. These are the temporary and permanent best management practices.
- 25. <u>✓</u> Locations where soil stabilization practices are expected to occur.
- 26. N/A Surface waters (including wetlands).
- 27. Locations where stormwater discharges to surface water or sensitive features.

 There will be no discharges to surface water or sensitive features.

ADMINISTRATIVE INFORMATION

- 28. ✓ One (1) original and four (4) copies of the completed application have been provided.
- 29. Any modification of this WPAP will require Executive Director approval, prior to construction, and may require submission of a revised application, with appropriate fees.

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **WATER POLLUTION ABATEMENT PLAN APPLICATION FORM** is hereby submitted for TCEQ review and Executive Director approval. The form was prepared by:

Aaron K. Parenica, P.E.

Print Name of Customer/Agent

Signature of **Qustomer/**Agent

Date

ATTACHMENT A

FACTORS AFFECTING WATER QUALITY

FACTORS AFFECTING WATER QUALITY

The materials listed below are anticipated to be present on-site during construction and as such may present a potential pollutant source: (This is not an all inclusive list).

- 1. Concrete/Masonry
- 2. Metal studs, Metal reinforcing bars, etc.
- 3. Tar
- 4. Fertilizers
- 5. Petroleum based products
- 6. Cleaning solvents/Detergents
- 7. Wood

Material management practices will be utilized to reduce the risk of spills, or other accidental exposure of the materials listed above to storm water runoff, including the following:

- 1. An effort shall be made to store only enough product required to complete the work as so defined in the approved construction documents.
- 2. All materials stored on-site shall be stored in a neat, orderly manner in their appropriate containers and, if possible, under a roof or other enclosure.
- 3. Products should be kept in their original containers with the original manufacturer's label.
- 4. Manufactures' recommendations for proper use and disposal shall be followed.
- 5. Substances shall not be mixed with one another unless recommended by the manufacturer.
- 6. Whenever possible, all of a product shall be used before disposing of its respective container.
- 7. The site superintendent should inspect daily to ensure proper use and disposal of on-site materials.

Post-Construction

The materials listed below are anticipated to be present on-site after construction and as such may present a potential pollutant source: (This is not an all inclusive list).

- 1. Vehicle Fluid and Petroleum based products (Motor Oil, Brake Fluid, Etc.)
- 2. Trash and Debris (Litter)
- 3. Discarded Food and Tobacco Products

These and other sources of pollutants which may affect storm water quality will be screened and filtered by proposed water quality ponds that will treat the storm water prior to releasing into the creek. All ponds will undergo periodic maintenance and cleaning to keep the integrity and effectiveness of treatment efficiency.

ATTACHMENT B

VOLUME AND CHARACTER OF STORM WATER

VOLUME AND CHARACTER OF STORM WATER

There are two existing drainage areas which fully encompass this site and total ± 64.88 acres. Using the City of New Braunfels runoff coefficients and incorporating their K-value into the equation, the two existing drainage areas will produce a peak flow of 161 Cubic Feet per Second (cfs) during a 100-year storm event. This existing watershed releases into two existing drainage easements located adjacent to the property, within the Hunters Creek Subdivision. The proposed drainage areas consist of three areas totaling ± 53.82 acres. Due to the City of New Braunfels requirements specific to this project, the combined total release rate stemming from the proposed drainage areas can not exceed 150 cfs for the 100-year storm event. All runoff stemming from the drainage areas will be captured and treated by three proposed water quality ponds and detained such that all City of New Braunfels requirements are met.

EXISTING CONDITIONS:

Drainage Area	K	С	I	A	Q100
DA-1	1.25	0.36	4.46	30.50	61.21
DA-2	1.25	0.36	6.45	34.38	99.79

ATTACHMENT C

SUITABILITY LETTER FROM AUTHORIZED AGENT (Not Applicable)

ATTACHMENT D

EXCEPTION TO THE REQUIRED GEOLOGIC ASSESSMENT (Not Applicable)

TEMPORARY STORM WATER SECTION

Temporary Stormwater Section

for Regulated Activities on the Edwards Aquifer Recharge Zone and Relating to 30 TAC §213.5(b)(4)(A), (B), (D)(I) and (G); Effective June 1, 1999

REGULATED ENTITY NAME: THE ENCLAVE AT WESTPOINTE VILLAGE

POTENTIAL SOURCES OF CONTAMINATION

Examples: Fuel storage and use, chemical storage and use, use of asphaltic products, construction vehicles tracking onto public roads, and existing solid waste.

venicie	es tracki	ing onto public roads, and existing solid waste.
1.	Fuels constru	for construction equipment and hazardous substances which will be used during uction:
	_ _ _ <u>✓</u>	Aboveground storage tanks with a cumulative storage capacity of less that 250 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity between 250 gallons and 499 gallons will be stored on the site for less than one (1) year. Aboveground storage tanks with a cumulative storage capacity of 500 gallons or more will be stored on the site. An Aboveground Storage Tank Facility Plan application must be submitted to the appropriate regional office of the TCEQ prior to moving the tanks onto the project. Fuels and hazardous substances will not be stored on-site.
2.	<u>✓</u>	ATTACHMENT A - Spill Response Actions . A description of the measures to be taken to contain any spill of hydrocarbons or hazardous substances is provided at the end of this form.
3.	N/A	Temporary aboveground storage tank systems of 250 gallons or more cumulative storage capacity must be located a minimum horizontal distance of 150 feet from any domestic, industrial, irrigation, or public water supply well, or other sensitive feature.
4.	<u>✓</u>	ATTACHMENT B - Potential Sources of Contamination. Describe in an attachment at the end of this form any other activities or processes which may be a potential source of contamination. There are no other potential sources of contamination.
SEQU	ENCE	OF CONSTRUCTION
5.	<u>✓</u>	ATTACHMENT C - Sequence of Major Activities. A description of the sequence of major activities which will disturb soils for major portions of the site (grubbing, excavation, grading, utilities, and infrastructure installation) is provided at the end of this form. For each activity described, an estimate of the total area of the site to be disturbed by each activity is given.
6.	\checkmark	Name the receiving water(s) at or near the site which will be disturbed or which will receive discharges from disturbed areas of the project: Blieders Creek

TEMPORARY BEST MANAGEMENT PRACTICES (TBMPs)

Erosion control examples: tree protection, interceptor swales, level spreaders, outlet stabilization, blankets or matting, mulch, and sod. Sediment control examples: stabilized construction exit, silt fence, filter dikes, rock berms, buffer strips, sediment traps, and sediment basins. Please refer to the Technical Guidance Manual for guidelines and specifications. All structural BMPs must be shown on the site plan.

- 7. ATTACHMENT D Temporary Best Management Practices and Measures. A description of the TBMPs and measures that will be used during and after construction are provided at the end of this form. For each activity listed in the sequence of construction, include appropriate control measures and the general timing (or sequence) during the construction process that the measures will be implemented.
 - TBMPs and measures will prevent pollution of surface water, groundwater, and stormwater. The construction-phase BMPs for erosion and sediment controls have been designed to retain sediment on site to the extent practicable. The following information has been provided in the attachment at the end of this form
 - a. A description of how BMPs and measures will prevent pollution of surface water, groundwater or stormwater that originates upgradient from the site and flows across the site.
 - b. A description of how BMPs and measures will prevent pollution of surface water or groundwater that originates on-site or flows off site, including pollution caused by contaminated stormwater runoff from the site.
 - c. A description of how BMPs and measures will prevent pollutants from entering surface streams, sensitive features, or the aquifer.
 - d. A description of how, to the maximum extent practicable, BMPs and measures will maintain flow to naturally-occurring sensitive features identified in either the geologic assessment, TCEQ inspections, or during excavation, blasting, or construction.
- 8. The temporary sealing of a naturally-occurring sensitive feature which accepts recharge to the Edwards Aquifer as a temporary pollution abatement measure during active construction should be avoided.
 - <u>N/A</u> ATTACHMENT E Request to Temporarily Seal a Feature. A request to temporarily seal a feature is provided at the end of this form. The request includes justification as to why no reasonable and practicable alternative exists for each feature.
 - ✓ There will be no temporary sealing of naturally-occurring sensitive features on the site.
- 9. ATTACHMENT F Structural Practices. Describe the structural practices that will be used to divert flows away from exposed soils, to store flows, or to otherwise limit runoff discharge of pollutants from exposed areas of the site. Placement of structural practices in floodplains has been avoided.

 \checkmark 10 ATTACHMENT G - Drainage Area Map. A drainage area map is provided at the end of this form to support the following requirements. \checkmark For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin will be provided. For areas that will have more than 10 acres within a common drainage area disturbed at one time, a smaller sediment basin and/or sediment trap(s) will be For areas that will have more than 10 acres within a common drainage area disturbed at one time, a sediment basin or other equivalent controls are not attainable, but other TBMPs and measures will be used in combination to protect down slope and side slope boundaries of the construction area. There are no areas greater than 10 acres within a common drainage area that will be disturbed at one time. A smaller sediment basin and/or sediment trap(s) will be used in combination with other erosion and sediment controls within each disturbed drainage area. 11. ATTACHMENT H - Temporary Sediment Pond(s) Plans and Calculations. Temporary sediment pond or basin construction plans and design calculations for a proposed temporary BMP or measure has been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information must be signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed temporary BMPs and measures are provided as at the end of this form. \checkmark 12. ATTACHMENT I - Inspection and Maintenance for BMPs. A plan for the inspection of temporary BMPs and measures and for their timely maintenance, repairs, and, if necessary, retrofit is provided at the end of this form. A description of documentation procedures and recordkeeping practices is included in the plan. 13 All control measures must be properly selected, installed, and maintained in accordance with the manufacturer's specifications and good engineering practices. If periodic inspections by the applicant or the executive director, or other information indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations. 14. If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize offsite impacts to water quality (e.g., fugitive sediment in street being washed into surface streams or sensitive features by the next rain). 15. Sediment must be removed from sediment traps or sedimentation ponds not later than when design capacity has been reduced by 50%. A permanent stake will be provided that can indicate when the sediment occupies 50% of the basin volume. 16. Litter, construction debris, and construction chemicals exposed to stormwater shall be

prevented from becoming a pollutant source for stormwater discharges (e.g., screening

outfalls, picked up daily).

SOIL STABILIZATION PRACTICES

Examples: establishment of temporary vegetation, establishment of permanent vegetation, mulching, geotextiles, sod stabilization, vegetative buffer strips, protection of trees, or preservation of mature vegetation.

- 17.

 ATTACHMENT J Schedule of Interim and Permanent Soil Stabilization Practices. A schedule of the interim and permanent soil stabilization practices for the site is attached at the end of this form.
- 18. Records must be kept at the site of the dates when major grading activities occur, the dates when construction activities temporarily or permanently cease on a portion of the site, and the dates when stabilization measures are initiated.
- 19. Stabilization practices must be initiated as soon as practicable where construction activities have temporarily or permanently ceased.

ADMINISTRATIVE INFORMATION

- 20. All structural controls will be inspected and maintained according to the submitted and approved operation and maintenance plan for the project.
- 21. If any geologic or manmade features, such as caves, faults, sinkholes, etc., are discovered, all regulated activities near the feature will be immediately suspended. The appropriate TCEQ Regional Office shall be immediately notified. Regulated activities must cease and not continue until the TCEQ has reviewed and approved the methods proposed to protect the aquifer from any adverse impacts.
- 22. Silt fences, diversion berms, and other temporary erosion and sediment controls will be constructed and maintained as appropriate to prevent pollutants from entering sensitive features discovered during construction.

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **TEMPORARY STORMWATER SECTION** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Aaron K. Parenica, P.E.

Print Name of Customer/Agent

Signature of Customer/Agent

Date

13-ZAID

ATTACHMENT A

SPILL RESPONSE ACTIONS

SPILL RESPONSE ACTIONS

Potential Source:

Spills of Hydrocarbons or other hazardous substances and materials.

Preventative Measures:

The following practices will be used to reduce the risks associated with hazardous materials, if hazardous materials are needed for the work:

Education/General Measures

- 1. Products will be kept in original containers unless they are not re-sealable.
- 2. Original labels and material safety data will be retained.
- 3. Modify the Storm Water Pollution Prevention Plan to include the information dealing with, and the steps needed to correct, the encountered hazardous waste spill.
- 4. Be aware that different materials pollute in different amounts. Make sure that each employee knows what a "significant spill" is for each material they use, and what is the appropriate response for "significant" and "insignificant" spills. Employees should also be aware of when spill must be reported to the TCEQ. Information available in 30 TAC 327.4 and 40 CFR 302.4.
- 5. Educate employees and subcontractors on potential dangers to humans and the environment from spills and leaks.
- 6. Hold regular meetings to discuss and reinforce appropriate disposal procedures (incorporate into regular safety meetings).
- 7. Establish a continuing education program to indoctrinate new employees.
- 8. Have contractor's superintendent or representative oversee and enforce proper spill prevention and control measures.
- 9. To the extent that the work can be accomplished safely, spills of oil, petroleum products, and substances listed under 40 CFR parts 110,117, and 302, as well as sanitary and septic wastes should be contained and cleaned up immediately.
- 10. Store hazardous materials and wastes in covered containers and protect from vandalism.

- 11. Place a stockpile of spill cleanup materials where it will be readily accessible.
- 12. Train employees in spill prevention and cleanup.
- 13. Designate responsible individuals to oversee and enforce control measures.
- 14. Spills should be covered and protected from storm water run-on during rainfall to the extent that it doesn't compromise clean up activities.
- 15. Do not bury or wash spills with water.
- 16. Store and dispose of used clean up materials, contaminated materials, and recovered spill material that is no longer suitable for the intended purpose in conformance with the provisions in applicable BMPs.
- 17. Do not allow water used for cleaning and decontamination to enter storm drains or watercourses. Collect and dispose of contaminated water in accordance with applicable regulations.
- 18. Contain water overflow or minor water spillage and do not allow it to discharge into drainage facilities or watercourses.
- 19. Place Material Safety Data Sheets (MSDS), as well as proper storage, cleanup, and spill reporting instructions for hazardous materials stored or used on the project site in an open, conspicuous, and accessible location.
- 20. Keep waste storage areas clean, well organized, and equipped with ample cleanup supplies as appropriate for the materials being stored. Perimeter controls, containment structures, covers, and liners should be repaired or replaced as needed to maintain proper function.

If surplus product must be disposed of, manufacturers' or local and state recommended methods for proper disposal will be followed.

Spill Measures:

In the event that hazardous wastes are encountered, they will be disposed of in the manner specified by local or state regulations.

Cleanup

- 1. Clean up leaks and spills immediately.
- 2. Use a rag for small spills on paved surfaces, a damp mop for general cleanup, and absorbent material for larger spills. If the spilled material is hazardous, then the used cleanup materials are also hazardous and must be disposed of as hazardous waste.
- 3. Never hose down or bury dry material spills. Clean up as much of the material as possible and dispose of properly. See the waste management BMPs in this section for specific information.

Minor Spills

- 1. Minor spills typically involve small quantities of oil, gasoline, paint, etc. which can be controlled by the first responder at the discovery of the spill.
- 2. Use absorbent materials on small spills rather than hosing down or burying the spill.
- 3. Absorbent materials should be promptly removed and disposed of properly.
- 4. Follow the practice below for a minor spill:
- 5. Contain the spread of the spill.
- 6. Recover spilled materials.
- 7. Clean the contaminated area and properly dispose of contaminated materials.

Semi-Significant Spills

Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. T his response may require the cessation of all other activities.

Spills should be cleaned up immediately

- 1. Contain spread of the spill.
- 2. Notify the project foreman immediately.
- 3. If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- 4. If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- 5. If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

Significant/Hazardous Spills

Spills of hazardous waste in amounts that equal or exceed Reportable Quantity (RQ), as defined by the EPA through issued regulations (40 CFR Part 110, 40 CFR Part 117, 40 CFR Part 119 or 40 CFR Part 302), will be handled in the following steps:

- 1. Notify the National Response Center immediately at 1-800-424-8802.
- 2. Notify TCEQ immediately at 1-210-490-3096 between 8 AM and 5 PM. After hours, contact the Environmental Release Hotline at 1-800-832-8224. It is the contractor's responsibility to have all emergency phone numbers at the construction site.
- 3. Submit a written description of the release to the EPA Region 11 office providing the date and circumstances of the release and the steps to be taken to prevent another release:

Attn: Hazardous Waste Dept. 1445 Ross Ave. STE 1200 Dallas, TX 75202 1-214-665-2224 (Region 6 Emergency Line)

- 4. The services of a spills contractor or a Haz-Mat team should be obtained immediately. Construction personnel should not attempt to clean up until the appropriate and qualified staffs have arrived at the job site.
- 5. Other agencies which may need to be consulted include, but are not limited to, the City Police Department, County Sheriff Office, Fire Departments, etc.

More information on spill rules and appropriate responses is available on the TCEQ website at: http://www.tceq.state.tx.us/response/html.

Vehicle Measures:

Vehicle and Equipment Maintenance

- 1. If maintenance must occur onsite, use a designated area and a secondary containment, located away from drainage courses, to prevent the run-on of stormwater and the runoff of spills.
- 2. Regularly inspect onsite vehicles and equipment for leaks and repair immediately.
- 3. Check incoming vehicles and equipment (including delivery trucks, and employee and subcontractor vehicles) for leaking oil and fluids. Do not allow leaking vehicles or equipment onsite.
- 4. Always use secondary containment, such as a drain pan or drop cloth, to catch spills or leaks when removing or changing fluids.
- 5. Place drip pans or absorbent materials under paving equipment when not in use.
- 6. Use absorbent materials on small spills rather than hosing down or burying the spill. Remove the absorbent materials promptly and dispose of properly.
- 7. Promptly transfer used fluids to the proper waste or recycling drums. Don't leave full drip pans or other open containers lying around.
- 8. Oil filters disposed of in trashcans or dumpsters can leak oil and pollute storm water. Place the oil filter in a funnel over a waste oil-recycling drum to drain excess oil before disposal. Oil filters can also be recycled. Ask the oil supplier or recycler about recycling oil filters.
- 9. Store cracked batteries in a non-leaking secondary container. Do this with all cracked batteries even if you think all the acid has drained out. If you drop a battery, treat it as if it is cracked. Put it into the containment area until you are sure it is not leaking.

Vehicle and Equipment Fueling

- 1. If fueling must occur on site, use designated areas, located away from drainage courses, to prevent the run-on of storm water and the runoff of spills.
- 2. Discourage "topping off" of fuel tanks.
- 3. Always use secondary containment, such as a drain pan, when fueling to catch spills/leaks.

ATTACHMENT B

POTENTIAL SOURCES OF CONTAMINATION

POTENTIAL SOURCES OF CONTAMINATION

Potential Source: Oil, grease, fuel and hydraulic fluid contamination from

construction equipment and vehicle dripping.

Preventative Measures: Vehicle maintenance when possible will be performed within the

construction staging area or at a local maintenance shop.

Potential Source: Miscellaneous trash and litter from construction workers and

material wrappings.

Preventative Measures: Trash containers will be placed throughout the site to encourage

proper trash disposal.

Potential Source: Construction debris.

Preventative Measures: Construction debris will be monitored daily by contractor.

Debris will be collected and placed in disposal bins. Situations requiring immediate attention will be addressed on a case-by-case

basis.

Potential Source: Silt leaving the site.

Preventative Measures: Contractor will monitor all vehicles leaving the site to prevent

tracking silt and mud onto public streets. The contractor will ensure that trucks will be washed down to minimize the amount

of silt leaving the site.

Potential Source: Connection to existing sewer line.

Preventative Measures: Contractor shall tie into existing sewer line per NBU regulations

and standards via a sanitary sewer manhole. A manhole detail is provided by NBU and shown on the construction details. Any leakage of sewage from the existing waste water line due to the

connection will be cleaned up immediately.

Potential Source: Construction related portable toilets.

Pre-Measures: Any on-site portable toilets will be in good working order with no

defects that cause leaks. All portable toilets will be maintained to

ensure no overflowing of sewage.

ATTACHMENT C

SEQUENCE OF MAJOR ACTIVITIES

SEQUENCE OF MAJOR ACTIVITIES

The sequence of work described below will be accomplished through the timing of proposed work relating the maintenance of service (i.e. proposed utility installation as compared to the removal/abandonment of existing utilities). Below is a general sequence of events to be followed:

- 1. Obtain all required permits.
- 2. Install all Erosion Control Measures and Devices that can be installed prior to site clearing. (±48.18 acres)
- 3. Clear site for streets and ponds. $(\pm 13.20 \text{ acres})$
- 4. Install any remaining Control Measures and Devices that could not be installed prior to site clearing.
- 5. Grade site and construct temporary sedimentation pond. Install Erosion Control around eatch basins. (±13.20 acres)
- 6. Set Sewage Collection System manholes and install all underground utilities and piping.
- 7. Install pavement (± 8.85 acres).
- 8. Inspect and maintain all erosion control measures until all disturbed offsite and on-site areas have been hydromulched or sodded in accordance with the landscape plan and a mowable stand of grass is achieved.

Total Site Area/Total Disturbed Area

The total area of the site is ± 48.18 acres. Excavation, grading, or other activities throughout the construction process will disturb approximately ± 31.37 acres. Post-construction impervious coverage will total ± 31.37 acres.

ATTACHMENT D

TEMPORARY BEST MANAGEMENT PRACTICES AND MEASURES

TEMPORARY BMPs

At the beginning of the project, Temporary Best Management Practices (BMPs) will be installed according to the attached Temporary BMP Details and placed as shown on the TBMP Site Plan.

Upgradient Water

The site is located due west from the southwest corner of the State Highway 46 and Loop 337 intersection, south of Oak Run parkway. Upgradient water from undeveloped sites upstream of the proposed development will be captured into a storm sewer system and routed northward to a proposed water quality and detention pond upon their completion. Prior to completion, upgradient water will be captured by the Temporary Sedimentation Basin.

On-site Water

Silt fencing will be placed along the boundary line of the majority of the tract. Inlet protection and triangular filter dikes will be placed as necessary to protect the areas affected by the Sewage Collection System (SCS) and its construction. These Temporary BMPs will be installed along the down-gradient boundary of the property to filter all runoff that originates on site and sequenced as indicated in the report. A temporary construction entrance will be installed to prevent tracking materials offsite. In addition, a concrete truck washout pit will be placed on-site and be accessible to all exiting traffic leaving the site. By this, the Temporary BMPs will prevent pollution of surface water that originates on-site due to trenching and bedding of the SCS. A Temporary Sedimentation Basin will be installed to treat runoff going into the existing drainage easements north of the site. Calculations for the Temporary Sedimentation Basin have been provided in Attachment H.

ATTACHMENT E

REQUEST TO TEMPORARILY SEAL A FEATURE (Not Applicable)

ATTACHMENT F

STRUCTURAL PRACTICES

STRUCTURAL PRACTICES

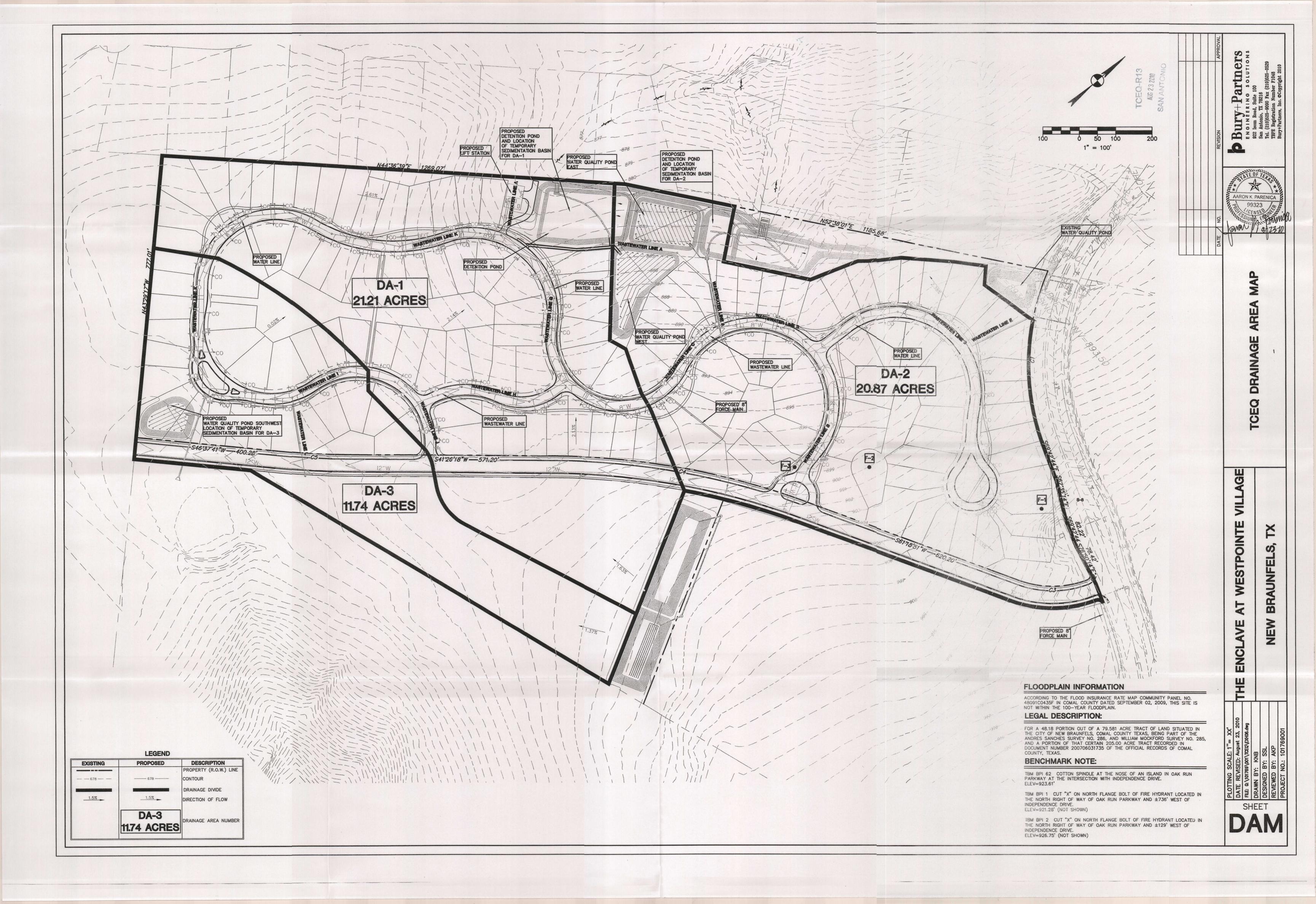
Silt fencing, triangular sediment filter dikes, inlet protection devices, and stabilized construction entrances will be incorporated as temporary erosion control devices and will be removed after permanent stabilization is established.

Silt fencing shall be incorporated throughout the construction process. The placement of the silt fencing shall be perpendicular to runoff flow. Refer to project construction documents for quantity and actual locations of these erosion control devices. In areas where silt fencing is to be situated but is non-installable, triangular filter dikes shall be incorporated.

Stabilized construction entrances will be employed during the construction of this development to help minimize vehicle tracking of sediments. Paved streets adjacent to these site entrances shall be cleaned and/or swept regularly to remove any excess mud, dirt or rock tracked from the site. Refer to the project construction documents for actual locations of these erosion control devices. Staging areas will be utilized in locations as decided by the project general contractor and validated by the civil engineer. If the contractor determines the need for additional stabilized construction entrances, construction staging areas or pits, their locations shall be agreed upon by the contractor and the engineer and annotated in the Storm Water Pollution Prevention Plan (SWPPP) posted on the site during construction.

ATTACHMENT G

DRAINAGE AREA MAP



ATTACHMENT H

TEMPORARY SEDIMENT POND(S)
PLANS AND CALCULATIONS

TEMPORARY SEDIMENTATION BASIN NOTES:

- 1. CONTRACTOR TO CONSTRUCT BASINS IN ACCORDANCE WITH CONSTRUCTION PLANS FOR PERMANENT SEDIMENTATION/FILTRATION WITH THE EXCEPTION OF THE GRAVEL DRAIN LAYER AND SAND FILTER LAYERS.
- 2. INSTALL PERMANENT STAKE TO INDICATE SEDIMENT LEVEL IN THE BASIN. STAKE SHOULD BE MARKED TO INDICATE WHEN SEDIMENT OCCUPIES 50% OF THE VOLUME OF THE BASIN
- 3. SEDIMENT WILL BE REMOVED WHEN MORE THAN 50% OF THE BASIN CAPACITY IS EXCEDDED.
- 4. CONTRACTOR MAY USE SEED IMPREGNATED STRAW MATTING FOR SLOPE STABILIZATION. MATTING MATERIAL TO BE APPROVED BY ENGINEER.
- 5. CONTRACTOR TO SECURE PIPE TO BOTTOM OF BASIN TO PREVENT BUOYANCY DURING A RAIN EVENT. A CONCRETE ANCHOR MAY BE USED.
- 6. DISCHARGE PIPE TO BE INSTALLED SO AS TO BE IN PLACE FOR PERMANENT STRUCTURE.

TEMPORARY SEDIMENTATION BASIN CALCULATIONS:

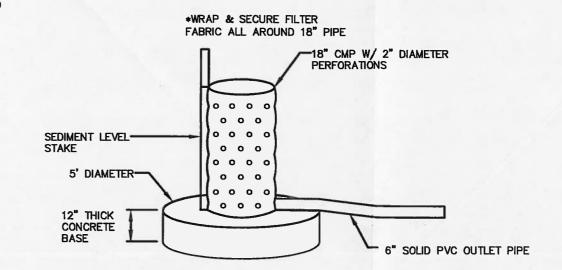
- 1. THE TEMPORARY SEDIMENTATION BASIN WILL BE LOCATED WHERE THE PROPOSED WATER QUALITY AND DETENTION POND(S) WILL BE CONSTRUCTED.
- 2. TOTAL DISTURBED AREA IS 31.37 ACRES
- 3. 31.37 AC \times (3,600 CF VOLUME / ACRE) = 112,932 CF OF WATER VOLUME TO BE CAPTURED IN THE TEMPORARY SEDIMENTATION BASIN
- 4. USING THE CAPACITY OF THE THREE PROPOSED WATER QUALITY PONDS:

SW 50,272 CF

W 63,830 CF

E 59,486 CF

5. THE TOTAL VOLUME AVAILABLE FROM ALL THREE TEMPORARY SEDIMENTATION BASINS IS 173,588 CF., THEREFORE TOTAL VOLUME IS ADEQUATE FOR THE TEMPORARY SEDIMENTATION BASINS.



R0101769-50001

L B M

VILLAGE

E AT WESTPOINTE BRAUNFELS, TEXA

ENCLAVE

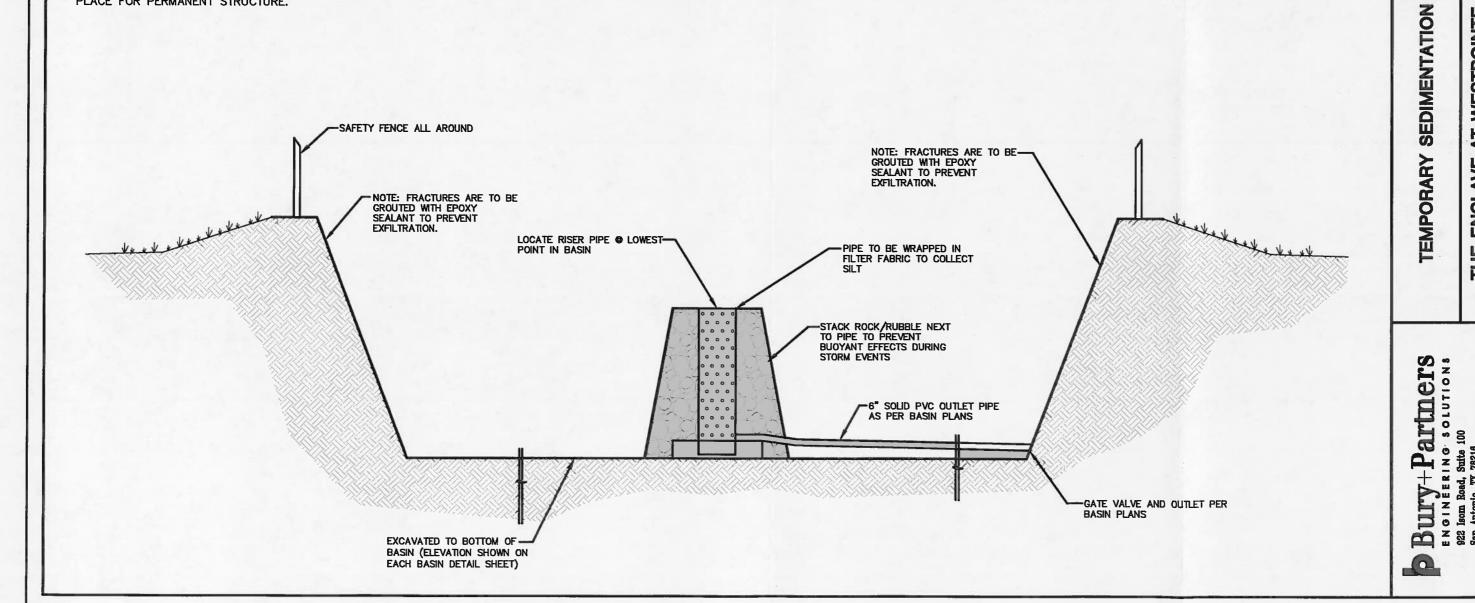
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(210)525-0529 ©Copyright 2010

DATE: AUGUST 2010

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BASIN



ATTACHMENT I

INSPECTION AND MAINTENANCE FOR BMP'S

INSPECTIONS

Each contractor will designate a qualified person (or persons) to perform the following inspections:

- 1. Disturbed areas and areas used for storage of materials that are exposed to precipitation will be inspected for evidence of, or the potential for, pollutants entering the drainage system.
- 2. Erosion and sediment control measures identified in the plan will be observed to ensure that they are operating correctly.
- 3. Where discharge locations or points are accessible, they will be inspected to ascertain whether erosion control measures are effective in preventing significant impacts to receiving waters.
- 4. Locations where vehicles enter or exit the site will be inspected for evidence of offsite sediment tracking.
- 5. Permanent seeding and planting will be inspected for bare spots, washouts and unhealthy growth.

The inspection shall be conducted by the responsible person at least once every seven (7) calendar days and within 24 hours after a storm providing 1/2 inches of rainfall or greater. If one or more of the following conditions apply, the frequency of inspections shall be conducted at least once every month:

- 1. The site has been either finally or temporarily stabilized.
- 2. Where runoff is unlikely due to winter conditions (i.e. site is covered with snow, ice, or where frozen ground exists.
- 3. During seasonal arid periods in arid areas (areas with an average annual rainfall of 0 to 10 inches) and semi-arid areas (areas with an average annual rainfall of 10 to 20 inches).

The information required within an inspection and maintenance report are as follows:

- 1. Summary of the scope of the inspection.
- 2. Name(s) and qualifications of personnel making the inspection.
- 3. The date(s) of the inspection.

- 4. Major observations relating to the implementation of the storm water pollution prevention plan.
- 5. Changes required to correct damages or deficiencies in the control measures.

In addition to the required routine inspections, the following record of information will also be maintained:

- 1. The dates when major sewer construction/and grading activities occur.
- 2. The dates when construction activities temporarily or permanently cease on a portion of the site.
- 3. The dates when stabilization measures are initiated.

Inspection and maintenance reports, as well as all records required by a Storm Water Pollution Prevention Plan (SWPPP), shall be included in the onsite SWPPP as part of the Texas Pollution Discharge Elimination System (TPDES) Report. Copies of example forms to be used for the inspection and maintenance reports along with their related records, will be included in the onsite SWPPP and are provided for reference.

MAINTENANCE

Based on the results of the inspection, any changes required to correct damages or deficiencies in the control measures shall be made within seven (7) calendar days after the inspection. If existing stabilization/erosion controls need modification or additional stabilization/erosion controls are necessary, implementation shall be achieved prior to the next anticipated storm event. If, however, the execution of this requirement becomes impractical, then the implementation will occur as soon as possible, with the incident duly noted with an explanation of the impracticality, in the inspection report.

Sediment accumulation at each control will be removed and properly disposed when the depth of accumulation equals or exceeds six (6) inches. If sediment accumulation is found to be contaminated, its disposal shall be off-site in a manner which conforms to the appropriate applicable regulations.

THE ENCLAVE AT WESTPOINTE VILLAGE

SW of Oak Run Pkwy, Near SH 46 and Loop 337 New Braunfels, Texas

Responsible Party Form Schedule

Prevention	Responsible Party Con	mpany Name
Pollution		
Measure		
Wicasure		
BEST MANAGEMENT PRACTICES		
Silt fences		
Rock berms		
Drain inlet protection		
Gravel filter bags		
Vehicle exits (offsite tracking)		
Concrete washout pit (leaks, failure)		
Temporary vegetation		
Permanent vegetation		
Sediment control basin		
Other structural controls		
Material storage areas (leakage)		
Equipment areas (leaks, spills)		
Construction debris		
General site cleanliness		
Trash receptacles		
Natural vegetation buffer strips		
Inspections		
SWP3 Modification & Records		
POTENTIAL EROSION SOURCES		
Clearing		
Grading		
Excavation		
Drainage Construction		
Utility Construction		
Roadway or Parking Lot Construction		
Foundation Construction		
Building Construction		
Landscaping Activities		

Identify responsible parties and indicate responsible party for each pollution prevention item listed above by marking an X under the Responsible Party Name.

THE ENCLAVE AT WESTPOINTE VILLAGE

SW of Oak Run Pkwy, Near SH 46 and Loop 337 New Braunfels, Texas

Inspection Report

Prevention	d in nce	Corrective Action Req	uired
Pollution	Inspected in Compliance	Description	Date
Measure	Insp	(use additional sheet if necessary)	Completed
	(Y/N)		
BEST MANAGEMENT PRACTICES			
Silt fences			
Rock berms			
Drain inlet protection			
Gravel filter bags			
Vehicle exits (offsite tracking)	A-9111-411-111-111-111-11-11-11-11-11-11-11		
Concrete washout pit (leaks, failure)			
Temporary vegetation			
Permanent vegetation			
Sediment control basin			
Other structural controls			
Material storage areas (leakage)			
Equipment areas (leaks, spills)			
Construction debris			
General site cleanliness			
Trash receptacles			
Natural vegetation buffer strips			
EVIDENCE OF EROSION			
Site preparation			
Roadway or Parking Lot Construction			
Utility Construction			
Drainage Construction			
Building Construction			
MAJOR OBSERVATIONS			
Sediment discharges from site			
BMPs requiring maintenance			
BMPs requiring modification			
Additional BMPs required			
"I certify under penalty of law that this document and all attac		· · · · · · · · · · · · · · · · ·	
assure that qualified personnel properly gather and evaluate th those persons directly responsible for gathering the informatio			
I am aware that there are significant penalties for submitting fa	•		
Inspector's Name (Superintendent)		Inspector's Signature	Date
Name of Owner/Operator (Firm)		Authorized Signature	Date

Note: If there is a "NO" answer in the second column, the right columns will need to be completed and action is required within 7 days. Use additional sheets if necessary.

ATTACHMENT J

SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION

SCHEDULE OF INTERIM AND PERMANENT SOIL STABILIZATION

During Construction:

The methodology for handling pollution of on-site or up-gradient storm water during construction will include the following:

- 1. Silt fencing and rock berms will be used as a temporary erosion and sedimentation controls.
- 2. Stabilized construction entrances/exits will be put into place to reduce the dispersion of sediment from the site, and to aid in accessibility to the site.
- 3. A construction staging area will also be put into place for material stockpiles, machinery storage, and machinery maintenance.
- 4. Concrete truck washout pits will be put into place to prevent contamination of storm water runoff and to aid in the removal of sediments from the site.
- 5. As required by the TCEQ General Permit, disturbed areas on which construction activity has ceased (temporarily or permanently) and which will be exposed for more than 21 days shall be stabilized within 14 days. Areas receiving less than 20 inches of annual rainfall should be stabilized as soon as practicable and only to pre-project conditions.
- 6. If construction stops for more than 14 days, hydro-seeding, sod or other TCEQ approved method will be applied to re-stabilize vegetation.

After Construction:

This site will provide the following permanent pollution abatement measures to prevent the pollution of storm water originating on-site or upgradient from the project site:

- 1. Storm water will be directed to grate inlets via curbing and grading and discharged into the sedimentation/filtration basins. The sedimentation/filtration basins have been designed to capture and filter the required runoff from the individual watersheds. The basin has been designed in accordance with the TCEQ Technical Guidance Manual. Each basin will be constructed as that particular phase is built.
- 2. Native grasses will be used on-site to help reduce the use of fertilizers and this will in turn reduce the levels of phosphates present in the stormwater runoff.
- 3. Where possible drainage will be directed across vegetated areas to provide some pretreatment prior to discharge into the filtration basin.

Permanent Erosion Control:

- 1. All disturbed areas shall be restored as noted below:
 - A minimum of 4" of topsoil shall be placed in all drainage channels (except rock) and between the curb and R.O.W. property lines.

2. Broadcast Seeding:

- From September 15 to March 1, seeding shall be with a combination of 2 pounds per 1,000 SF of unhulled Bermuda and 7 pounds per 1000 SF of Winter Rye with a purity of 95% with 90% germination.
- From March 2 to September 14, seeding shall be with hulled Bermuda at a rate of 2 pounds per 1000 SF with a purity of 95% with 85% germination.
- 3. Fertilizer shall be a pelleted or granular slow release with an analysis of 15-15-15 to be applied once at planting and once during the period of establishment at a rate of 1 pound per 1,000 SF.

4. Hydraulic Seeding:

- From September 15 to March 1, seeding shall be with a combination of 1 pound per 1,000 SF of unhulled Bermuda and 7 pounds per 1,000 SF of Winter Rye with a purity of 95% with 90% germination.
- From March 2 to September 14, seeding shall be with hulled Bermuda at a rate of 7 pounds per 1,000 SF with a purity of 95% with 85% germination.
- 5. Fertilizer shall be a water soluble fertilizer with an analysis of 15-15-15 at a rate of 1 to 1.5 pounds per 1,000 SF (45-65 pounds per acre).
- 6. Mulch type used shall be hay, straw, or mulch applied at a rate of 45 pounds per 1,000 SF with a soil tackifier at a rate of 1.4 pounds per 1,000 SF.
- 7. The planted area shall be irrigated or sprinkled in a manner that will not erode the topsoil but will sufficiently soak the soil to a depth of 6". The irrigation shall occur at ten-day intervals during the first two months. Rainfall occurrences of ½" or more shall postpone the watering schedule for one week.
- 8. Restoration shall be acceptable when the grass has grown at least 1½" high with 95% coverage, provided no bare spots larger than 16 square feet exist.

PERMANENT STORM WATER SECTION

Permanent Stormwater Section

for Regulated Activities
on the Edwards Aquifer Recharge Zone
and Relating to 30 TAC §213.5(b)(4)(C), (D)(Ii), (E), and (5), Effective June 1, 1999

REGL	LATED	NTITY NAME: THE ENCLAVE AT WESTPOINTE VILLAGE	
		t management practices (BMPs) and measures that will be used during a on is completed.	ınd
1.	<u> </u>	ermanent BMPs and measures must be implemented to control the discharge ollution from regulated activities after the completion of construction.	of
2.	✓	nese practices and measures have been designed, and will be constructed, operated maintained to insure that 80% of the incremental increase in the annual mading of total suspended solids (TSS) from the site caused by the regulated activity moved. These quantities have been calculated in accordance with technicidance prepared or accepted by the executive director.	ass y is
		The TCEQ Technical Guidance Manual (TGM) was used to design permane BMPs and measures for this site. A technical guidance other than the TCEQ TGM was used to design permane BMPs and measures for this site. The complete citation for the technical guidance that was used is provided below:	ent
3.	<u> </u>	wners must insure that permanent BMPs and measures are constructed and funct designed. A Texas Licensed Professional Engineer must certify in writing that ermanent BMPs or measures were constructed as designed. The certification left ust be submitted to the appropriate regional office within 30 days of site completion	the tter
4.	<u>✓</u>	There a site is used for low density single-family residential development and has or less impervious cover, other permanent BMPs are not required. This exempt om permanent BMPs must be recorded in the county deed records, with a notice to the percent impervious cover increases above 20% or land use changes, comption for the whole site as described in the property boundaries required by AC §213.4(g) (relating to Application Processing and Approval), may no longer append the property owner must notify the appropriate regional office of these changes.	tion hat the 30 oply
		This site will be used for low density single-family residential development a has 20% or less impervious cover. This site will be used for low density single-family residential development has more than 20% impervious cover.	

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must notify the appropriate regional office of these changes.

5.

This site will not be used for low density single-family residential development.

The executive director may waive the requirement for other permanent BMPs for multifamily residential developments, schools, or small business sites where 20% or less impervious cover is used at the site. This exemption from permanent BMPs must be recorded in the county deed records, with a notice that if the percent impervious cover increases above 20% or land use changes, the exemption for the whole site as described in the property boundaries required by 30 TAC §213.4(g) (relating to Application Processing and Approval), may no longer apply and the property owner

- N/A ATTACHMENT A 20% or Less Impervious Cover Waiver. This site will be used for multi-family residential developments, schools, or small business sites and has 20% or less impervious cover. A request to waive the requirements for other permanent BMPs and measures is found at the end of this form.
- N/A This site will be used for multi-family residential developments, schools, or small business sites but has more than 20% impervious cover.
- This site will not be used for multi-family residential developments, schools, or small business sites.

6. ATTACHMENT B - BMPs for Upgradient Stormwater.

- A description of the BMPs and measures that will be used to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site is identified as **ATTACHMENT B** at the end of this form.
- N/A If no surface water, groundwater or stormwater originates upgradient from the site and flows across the site, an explanation is provided as **ATTACHMENT B** at the end of this form.
- N/A If permanent BMPs or measures are not required to prevent pollution of surface water, groundwater, or stormwater that originates upgradient from the site and flows across the site, an explanation is provided as **ATTACHMENT B** at the end of this form.

7. ATTACHMENT C - BMPs for On-site Stormwater.

- A description of the BMPs and measures that will be used to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff from the site is identified as ATTACHMENT C at the end of this form.
- N/A If permanent BMPs or measures are not required to prevent pollution of surface water or groundwater that originates on-site or flows off the site, including pollution caused by contaminated stormwater runoff, an explanation is provided as **ATTACHMENT C** at the end of this form.
- 8. N/A ATTACHMENT D BMPs for Surface Streams. A description of the BMPs and measures that prevent pollutants from entering surface streams, sensitive features, or the aquifer is provided at the end of this form. Each feature identified in the Geologic Assessment as "sensitive" has been addressed
- 9. The applicant understands that to the extent practicable, BMPs and measures must maintain flow to naturally occurring sensitive features identified in either the geologic assessment, executive director review, or during excavation, blasting, or construction.
 - The permanent sealing of or diversion of flow from a naturally-occurring "sensitive" or "possibly sensitive" feature that accepts recharge to the Edwards Aquifer as a permanent pollution abatement measure has not been proposed for any naturally-occurring "sensitive" or "possibly sensitive" features on this site.
 - N/A ATTACHMENT E Request to Seal Features. A request to seal a naturally-occurring "sensitive" or "possibly sensitive" feature, that includes a justification as to why no reasonable and practicable alternative exists, is found at the end of this form. A request and justification has been provided for each feature.
- 10. ✓ ATTACHMENT F Construction Plans. Construction plans and design calculations for the proposed permanent BMPs and measures have been prepared by or under the direct supervision of a Texas Licensed Professional Engineer. All construction plans and design information have been signed, sealed, and dated by the Texas Licensed Professional Engineer. Construction plans for the proposed permanent BMPs and

TCEQ-0600 (Rev. 10/01/04) Page 2 of 3

measures are provided at the end of this form. Design Calculations, TCEQ Construction Notes, all man-made or naturally occurring geologic features, all proposed structural measures, and appropriate details must be shown on the construction plans.

- 11. ✓ ATTACHMENT G Inspection, Maintenance, Repair and Retrofit Plan. A plan for the inspection, maintenance, repair, and, if necessary, retrofit of the permanent BMPs and measures is provided at the end of this form. The plan has been prepared and certified by the engineer designing the permanent BMPs and measures. The plan has been signed by the owner or responsible party. The plan includes procedures for documenting inspections, maintenance, repairs, and, if necessary, retrofits as well as a discussion of record keeping procedures.
- 12. <u>✓</u> The TCEQ Technical Guidance Manual (TGM) was used to design permanent BMPs and measures for this site.
 - N/A Pilot-scale field testing (including water quality monitoring) may be required for BMPs that are not contained in technical guidance recognized by or prepared by the executive director.
 - <u>N/A</u> **ATTACHMENT H Pilot-Scale Field Testing Plan.** A plan for pilot-scale field testing is provided at the end of this form.
- 13. N/A ATTACHMENT I -Measures for Minimizing Surface Stream Contamination. A description of the measures that will be used to avoid or minimize surface stream contamination and changes in the way in which water enters a stream as a result of the construction and development is provided at the end of this form. The measures address increased stream flashing, the creation of stronger flows and in-stream velocities, and other in-stream effects caused by the regulated activity which increase erosion that results in water quality degradation.

Responsibility for maintenance of permanent BMPs and measures after construction is complete.

- 14.

 The applicant is responsible for maintaining the permanent BMPs after construction until such time as the maintenance obligation is either assumed in writing by another entity having ownership or control of the property (such as without limitation, an owner's association, a new property owner or lessee, a district, or municipality) or the ownership of the property is transferred to the entity. Such entity shall then be responsible for maintenance until another entity assumes such obligations in writing or ownership is transferred.
- A copy of the transfer of responsibility must be filed with the executive director at the appropriate regional office within 30 days of the transfer if the site is for use as a multiple single-family residential development, a multi-family residential development, or a non-residential development such as commercial, industrial, institutional, schools, and other sites where regulated activities occur.

To the best of my knowledge, the responses to this form accurately reflect all information requested concerning the proposed regulated activities and methods to protect the Edwards Aquifer. This **PERMANENT STORMWATER SECTION** is hereby submitted for TCEQ review and executive director approval. The application was prepared by:

Aaron K. Parenica, P.E.
Print Name of Customer/Agent

Signature of Customer Agent

Date

TCEQ-0600 (Rev. 10/01/04)

Page 3 of 3

ATTACHMENT A

20% OR LESS IMPERVIOUS COVER WAIVER (Not Applicable)

ATTACHMENT B

BMP'S FOR UPGRADIENT STORM WATER

BMPS FOR UPGRADIENT STORM WATER

The Permanent BMPs for this project will incorporate design for future land uses up-gradient from the site. Therefore, the drainage areas and TCC removal calculations incorporate such areas into the calculation for the three proposed Water Quality Ponds. As such, there are no other surface water, groundwater, nor storm water that originates up-gradient from the site that flow through or across the project site.

ATTACHMENT C

BMP'S FOR ON-SITE STORM WATER

BMPS FOR ON-SITE STORM WATER Three sand filter water quality ponds are proposed to prevent pollution of surface water or groundwater from runoff that originates on-site or flows off the site.

ATTACHMENT D

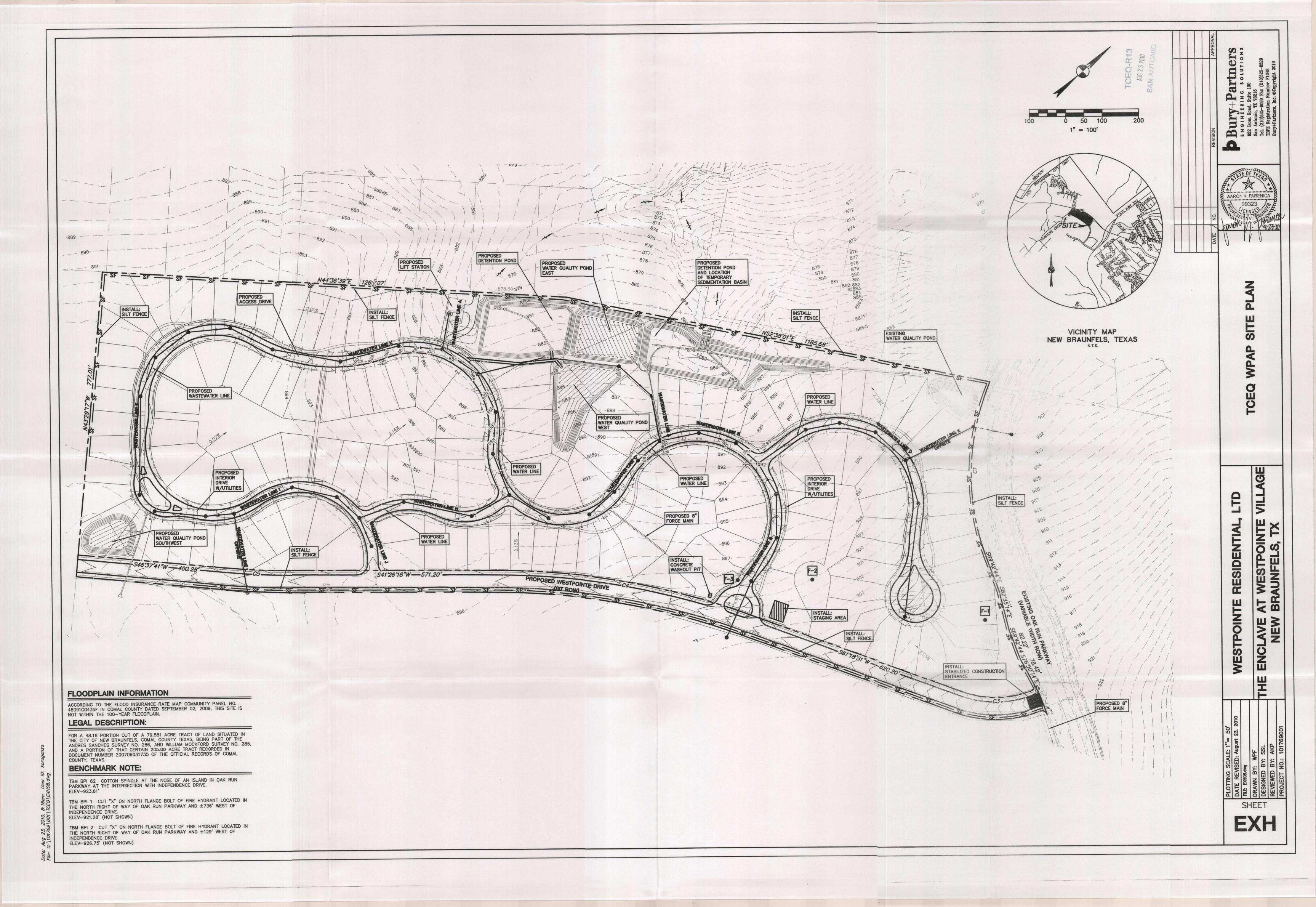
BMP'S FOR SURFACE STREAMS (Not Applicable)

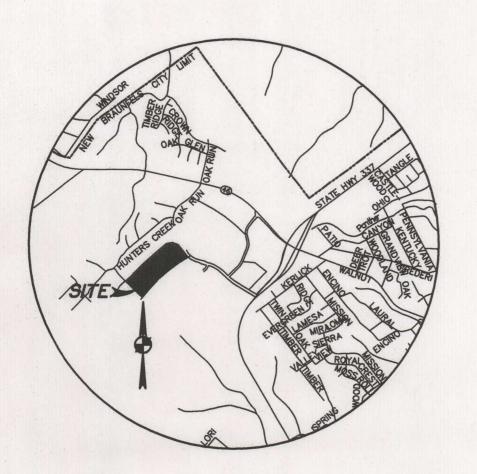
ATTACHMENT E

REQUEST TO TEMPORARILY SEAL A FEATURE (Not Applicable)

ATTACHMENT F

CONSTRUCTION PLANS





VICINITY MAP NEW BRAUNFELS. **TEXAS**

ENGINEER: BURY+PARTNERS, INC.

922 ISOM ROAD, SUITE 100 SAN ANTONIO, TEXAS 78216

(210) 525-9090

ATTN .: AARON K. PARENICA, P.E.

SURVEY PROVIDED BY: BURY+PARTNERS, INC.

922 ISOM ROAD, SUITE 100 SAN ANTONIO, TEXAS 78216

(210) 525-9090 ATTN .: HAL LANE, R.P.L.S.

BPI JOB NO: 0101769-01

THE ENCLAVE AT WESTPOINTE VILLAGE OAK RUN PKWY & WESTPOINTE DRIVE NEW BRAUNFELS, TEXAS

SHEET INDEX WATER OLIALITY DOND DI ANI

WATER QUALITY POND PLAN	SHEET NO.
COVER SHEET.	
WATER QUALITY POND PLAN 1.	C13.01
WATER QUALITY POND PLAN 2	
WATER QUALITY POND PLAN 3.	
WATER QUALITY POND DETAILS.	C13.04
WPAP CALCULATIONS.	

WATER QUALITY POND PLANS

TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

WATER POLLUTION ABATEMENT PLAN NOTES:

1. CONTRACTOR IS ADVISED THAT TCEQ DOES NOT ALLOW CHANGES TO PERMANENT POLLUTION ABATEMENT MEASURES WITHOUT THEIR PRIOR APPROVAL. 2. TOPS OF CLEANOUTS SHALL BE SET AT SIX INCHES ABOVE THE SAND ELEVATION.

3. SEDIMENT WILL BE REMOVED WHEN THE MATERIAL FILLS THE BASIN TO THE TOPS OF THE CLEANOUTS.

4. CONTRACTOR TO HYDROMULCH EARTHEN SLOPES FOR SLOPE STABILIZATION DURING INITIAL BASIN CONSTRUCTION AND MAINTAIN WATERING UNTIL VEGETATION IS FULLY ESTABLISHED.

5. AS AN ALTERNATE TO ITEM 4, CONTRACTOR MAY USE SEED IMPREGNATED STRAW MATTING FOR SLOPE STABILIZATION. MATTING MATERIAL TO BE APPROVED BY ENGINEER.

6. CONTRACTOR SHALL NOTIFY CERTIFYING ENGINEER WHEN BASIN CONSTRUCTION HAS PROGRESSED TO THE FOLLOWING MILESTONES:

- a.) BASIN LINER IN PLACE AND UNDER DRAIN SYSTEM IS IN PLACE WITHOUT
- b.) GRAVEL AROUND UNDER DRAIN SYSTEM IS IN PLACE AND FILTER FABRIC IS INSTALLED AND ATTACHED TO WALLS OR RIP-RAP.
- c.) SAND FILTER MEDIA HAS BEEN PLACED & BASIN HAS BEEN COMPLETELY FINISHED INCLUDING SOD OR SEED PLACEMENT ON SIDE SLOPES (WHERE

. WORK SHALL NOT CONTINUE ON THE BASIN UNTIL THE ENGINEER HAS HAD AN OPPORTUNITY TO OBSERVE THE STATUS OF CONSTRUCTION AT EACH STAGE. CONTRACTOR SHALL PROVIDE ENGINEER A MINIMUM OF 24 HOURS ADVANCE NOTICE PRIOR TO TIME THE BASIN WILL BE AT THE REQUIRED STAGE.

ENGINEER WITH FIELD SHOTS VERIFYING ELEVATIONS OF THE FOLLOWING: - TOP OF BANK AT EACH CORNER OF BASIN - TOE OF SLOPE AT EACH CORNER OF BASIN (INSIDE BASIN TOE) - SPLASH PAD/INLET PIPE - OVERFLOW WEIR

9. CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITY WITH STRUCTURAL ENGINEER SO AS TO PROVIDE APPROPRIATE OPPORTUNITY FOR STRUCTURAL ENGINEER TO MAKE THE NECESSARY CONSTRUCTION OBSERVATIONS DURING

10. CONTRACTOR SHALL CERTIFY TO THE ENGINEER THAT ALL STRUCTURES HAVE BEEN CONSTRUCTED TO THE DESIGN ELEVATIONS AT THE COMPLETION OF CONSTRUCTION. THIS SHALL INCLUDE ALL STRUCTURAL ELEMENTS, CONCRETE WALLS, BASIN INVERTS, TOP OF EARTH BASIN WALLS, TOP OF SAND ELEVATIONS, INVERTS OF SEDIMENTATION CHAMBER ELEVATIONS, AND ELEVATIONS OF INLET AND OVERFLOW DEVICES.

11. ALL UNDERDRAIN 4" & 6" PVC PIPE WITHIN WATER QUALITY POND TO BE

GENERAL CONSTRUCTION NOTES:

WRITTEN CONSTRUCTION NOTIFICATION MUST BE GIVEN TO THE APPROPRIATE TCEQ REGIONAL OFFICE NO LATER THAN 48 HOURS PRIOR TO COMMENCEMENT OF THE REGULATED ACTIVITY. INFORMATION MUST INCLUDE THE DATE ON WHICH HE REGULATED ACTIVITY WILL COMMENCE, THE NAME OF THE APPROVED PLAN FOR THE REGULATED ACTIVITY, AND THE NAME OF THE PRIME CONTRACTOR AND THE NAME AND TELEPHONE NUMBER OF THE CONTACT PERSON.

2. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROJECT MUST BE PROVIDED WITH COMPLETE COPIES OF THE APPROVED WATER POLLUTION ABATEMENT PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS ARE REQUIRED TO KEEP ON-SITE COPIES OF THE APPROVED PLAN AND APPROVAL LETTER.

3. IF ANY SENSITIVE FEATURE IS DISCOVERED DURING CONSTRUCTION, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPROPRIATE TCEQ REGIONAL OFFICE MUST BE IMMEDIATELY NOTIFIED OF ANY SENSITIVE FEATURES ENCOUNTERED DURING CONSTRUCTION. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE TCEQ HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY.

4. NO TEMPORARY ABOVEGROUND HYDROCARBON AND HAZARDOUS SUBSTANCE STORAGE TANK SYSTEM IS INSTALLED WITHIN 150 FEET OF A DOMESTIC, INDUSTRIAL, IRRIGATION, OR PUBLIC WATER SUPPLY WELL, OR OTHER SENSITIVE FEATURE.

5. ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY SELECTED, INSTALLED, AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS AND GOOD ENGINEERING PRACTICES. CONTROLS SPECIFIED IN THE TEMPORARY STORM WATER SECTION OF THE APPROVED EDWARDS AQUIFER PROTECTION PLAN ARE REQUIRED DURING CONSTRUCTION. IF INSPECTIONS INDICATE A CONTROL HAS BEEN USED INAPPROPRIATELY, OR INCORRECTLY, THE APPLICANT MUST REPLACE OR MODIFY THE CONTROL FOR SITE SITUATIONS. THE CONTROLS MUST REMAIN IN PLACE UNTIL DISTURBED AREAS ARE REVEGETATED AND THE AREAS HAVE BECOME PERMANENTLY STABILIZED.

6. IF SEDIMENT ESCAPES THE CONSTRUCTION SITE, OFF-SITE ACCUMULATIONS OF SEDIMENT MUST BE REMOVED AT A FREQUENCY SUFFICIENT TO MINIMIZE OFFSITE IMPACTS TO WATER QUALITY (E.G., FUGITIVE SEDIMENT IN STREET BEING WASHED INTO SURFACE STREAMS OR SENSITIVE FEATURES BY THE NEXT RAIN).

7. SEDIMENT MUST BE REMOVED FROM SEDIMENT TRAPS OR SEDIMENTATION PONDS NOT LATER THAN WHEN DESIGN CAPACITY HAS BEEN REDUCED BY 50%. A PERMANENT STAKE MUST BE PROVIDED THAT CAN INDICATE WHEN THE SEDIMENT OCCUPIES 50% OF

8. LITTER, CONSTRUCTION DEBRIS, AND CONSTRUCTION CHEMICALS EXPOSED TO STORMWATER SHALL BE PREVENTED FROM BECOMING A POLLUTANT SOURCE FOR STORMWATER DISCHARGES (E.G., SCREENING OUTFALLS, PICKED UP DAILY).

9. ALL SPOILS (EXCAVATED MATERIAL) GENERATED FROM THE PROJECT SITE MUST BE STORED ON-SITE WITH PROPER E&S CONTROLS. FOR STORAGE OR DISPOSAL OF SPOILS AT ANOTHER SITE ON THE EDWARDS AQUIFER RECHARGE ZONE, THE OWNER OF THE SITE MUST RECEIVE APPROVAL OF A WATER POLLUTION ABATEMENT PLAN FOR THE PLACEMENT OF FILL MATERIAL OR MASS GRADING PRIOR TO THE PLACEMENT OF SPOILS AT THE OTHER SITE.

10. STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE IN PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT IN NO CASE MORE THAN 14 DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT PORTION OF THE SITE HAS TEMPORARILY OR PERMANENTLY CEASED. WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER CONSTRUCTION ACTIVITY TEMPORARY OR PERMANENTLY CEASE IS PRECLUDED BY WEATHER CONDITIONS, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE. WHERE CONSTRUCTION ACTIVITY ON A PORTION OF THE SITE IS TEMPORARILY CEASED, AND EARTH DISTURBING ACTIVITIES WILL BE RESUMED WITHIN 21 DAYS, TEMPORARY STABILIZATION MEASURES DO NOT HAVE TO BE INITIATED ON THAT PORTION OF SITE. IN AREAS EXPERIENCING DROUGHTS WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER CONSTRUCTION ACTIVITY HAS TEMPORARILY OR PERMANENTLY CEASED IS PRECLUDED BY SEASONAL ARID CONDITIONS, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE.

11. THE FOLLOWING RECORDS SHALL BE MAINTAINED AND MADE AVAILABLE TO THE TCEQ UPON REQUEST: THE DATES WHEN MAJOR GRADING ACTIVITIES OCCUR; THE DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE; AND THE DATES WHEN STABILIZATION MEASURES ARE INITIATED. 12. THE HOLDER OF ANY APPROVED EDWARD AQUIFER PROTECTION PLAN MUST NOTIFY THE APPROPRIATE REGIONAL OFFICE IN WRITING AND OBTAIN APPROVAL FROM THE EXECUTIVE DIRECTOR PRIOR TO INITIATING ANY OF THE FOLLOWING:

A. ANY PHYSICAL OR OPERATIONAL MODIFICATION OF ANY WATER POLLUTION ABATEMENT STRUCTURE(S), INCLUDING BUT NOT LIMITED TO PONDS, DAMS, BERMS, SEWAGE TREATMENT PLANTS, AND DIVERSIONARY STRUCTURES;

B. ANY CHANGE IN THE NATURE OR CHARACTER OF THE REGULATED ACTIVITY FROM THAT WHICH WAS ORIGINALLY APPROVED OR A CHANGE WHICH WOULD SIGNIFICANTLY IMPACT THE ABILITY OF THE PLAN TO PREVENT POLLUTION OF THE EDWARDS AQUIFER:

C. ANY DEVELOPMENT OF LAND PREVIOUSLY IDENTIFIED AS UNDEVELOPED IN THE ORIGINAL WATER POLLUTION ABATEMENT PLAN.

SAN ANTONIO REGIONAL OFFICE 14250 JUDSON RD. SAN ANTONIO, TEXAS 78233-4480 PHONE (210) 490-3096 FAX (210) 545-4329

AUSTIN REGIONAL OFFICE 2800 S. IH-35, SUITE 100 AUSTIN, TEXAS 78704-5712 PHONE (512) 339-2929 FAX (512) 339-3795

SPECIAL CONSTRUCTION NOTES:

1. THE CONTRACTOR SHALL CONTACT THE CITY OF SAN ANTONIO PUBLIC WORKS DEPARTMENT AND ALL UTILITY COMPANIES LOCATOR 48 HOURS BEFORE BEGINNING ANY EXCAVATION. 2. DUE TO FEDERAL REGULATIONS TITLE 49, PART 192.181. CPS MUST MAINTAIN ACCESS TO GAS VALVES AT ALL TIMES. THE CONTRACTOR MUST PROTECT AND WORK AROUND ANY GAS VALVES THAT ARE IN THE PROJECT

3. THE EXISTENCE AND LOCATION OF UNDERGROUND CABLE INDICATED ON THE PLANS ARE TAKEN FROM THE BEST RECORDS AVAILABLE AND ARE NOT GUARANTEED TO BE ACCURATE. CONTRACTOR TO CONTACT THE TELEPHONE COMPANY CABLE LOCATOR 48 HOURS PRIOR TO EXCAVATION. CONTRACTOR HAS THE RESPONSIBILITY TO PROTECT AND SUPPORT TELEPHONE COMPANY PLANT DURING CONSTRUCTION.

4. THE CONTRACTOR SHALL CONTACT CITY OF SAN ANTONIO PUBLIC WORKS, WATER LINE LOCATOR 48 HOURS PRIOR TO EXCAVATION IN THE IMMEDIATE AREA OF WATER LINE. 5. DAMAGE TO ANY UNDERGROUND DRAINAGE SYSTEM SHALL BE REPORTED

TO CITY OF SAN ANTONIO PUBLIC WORKS FOR CONSULTATION WITH THE CITY'S DRAINAGE SUPERINTENDENT. THE SUPERINTENDENT WILL INSTRUCT THE DAMAGING PARTY (CONTRACTOR) ON HOW TO REPAIR THE LINE AT THE CONTRACTORS COST. 6. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING AND PROTECTING THE INTEGRITY OF THE POWER POLES DURING CONSTRUCTION. THE CONTRACTOR SHALL BE REQUIRED AT THEIR EXPENSE TO PROVIDE ACCEPTABLE BRACING

OF SPECIFIC C.P.S. UTILITIES POLES DURING THE CONSTRUCTION OF THIS PROJECT AND/OR PROVIDE AT THEIR EXPENSE FOR C.P.S. UTILITIES TO PROVIDE BRACING. IN ADDITION IT IS CRITICAL THE CONTRACTOR WORK CLOSELY WITH C.P.S. UTILITIES CONSTRUCTION FORMAN FOR THE SAKE OF SAFETY TO ISOLATE AND/OR PROTECT CONTRACTOR FROM ENERGIZED ELECTRIC CONDUCTORS ABOVE AREAS OF PROPOSED EXCAVATION.

TRENCH EXCAVATION SAFETY PROTECTION:

CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITE(S) WITHIN THE PROJECT AREA IN ORDER TO IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS PROGRAMS AND/OR PROCEDURES. THE CONTRACTOR'S IMPLEMENTATION OF THE SYSTEM'S PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLIES WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATION. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION.

FLOODPLAIN INFORMATION

ACCORDING TO THE FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 48091C0435F IN COMAL COUNTY DATED SEPTEMBER 02, 2009, THIS SITE IS NOT WITHIN THE 100-YEAR FLOODPLAIN.

LEGAL DESCRIPTION:

FOR A 48.18 PORTION OUT OF A 79.581 ACRE TRACT OF LAND SITUATED IN THE CITY OF NEW BRAUNFELS, COMAL COUNTY TEXAS, BEING PART OF THE ANDRES SANCHES SURVEY NO. 286, AND WILLIAM MOCKFORD SURVEY NO. 285, AND A PORTION OF THAT CERTAIN 205.00 ACRE TRACT RECORDED IN DOCUMENT NUMBER 200706031735 OF THE OFFICIAL RECORDS OF COMAL COUNTY, TEXAS.

BENCHMARK NOTE:

TBM BPI 62 COTTON SPINDLE AT THE NOSE OF AN ISLAND IN OAK RUN PARKWAY AT THE INTERSECTION WITH INDEPENDENCE DRIVE. ELEV=923.61'

TBM BPI 1 CUT "X" ON NORTH FLANGE BOLT OF FIRE HYDRANT LOCATED IN THE NORTH RIGHT OF WAY OF OAK RUN PARKWAY AND ±736' WEST OF INDEPENDENCE DRIVE. ELEV=921.28' (NOT SHOWN)

TBM BPI 2 CUT "X" ON NORTH FLANGE BOLT OF FIRE HYDRANT LOCATED IN THE NORTH RIGHT OF WAY OF OAK RUN PARKWAY AND ±129' WEST OF INDEPENDENCE DRIVE. ELEV=926.75' (NOT SHOWN)

TCEO-P13 AUG 23 2012

TCEO P13

SHEET REVISION APPROVA

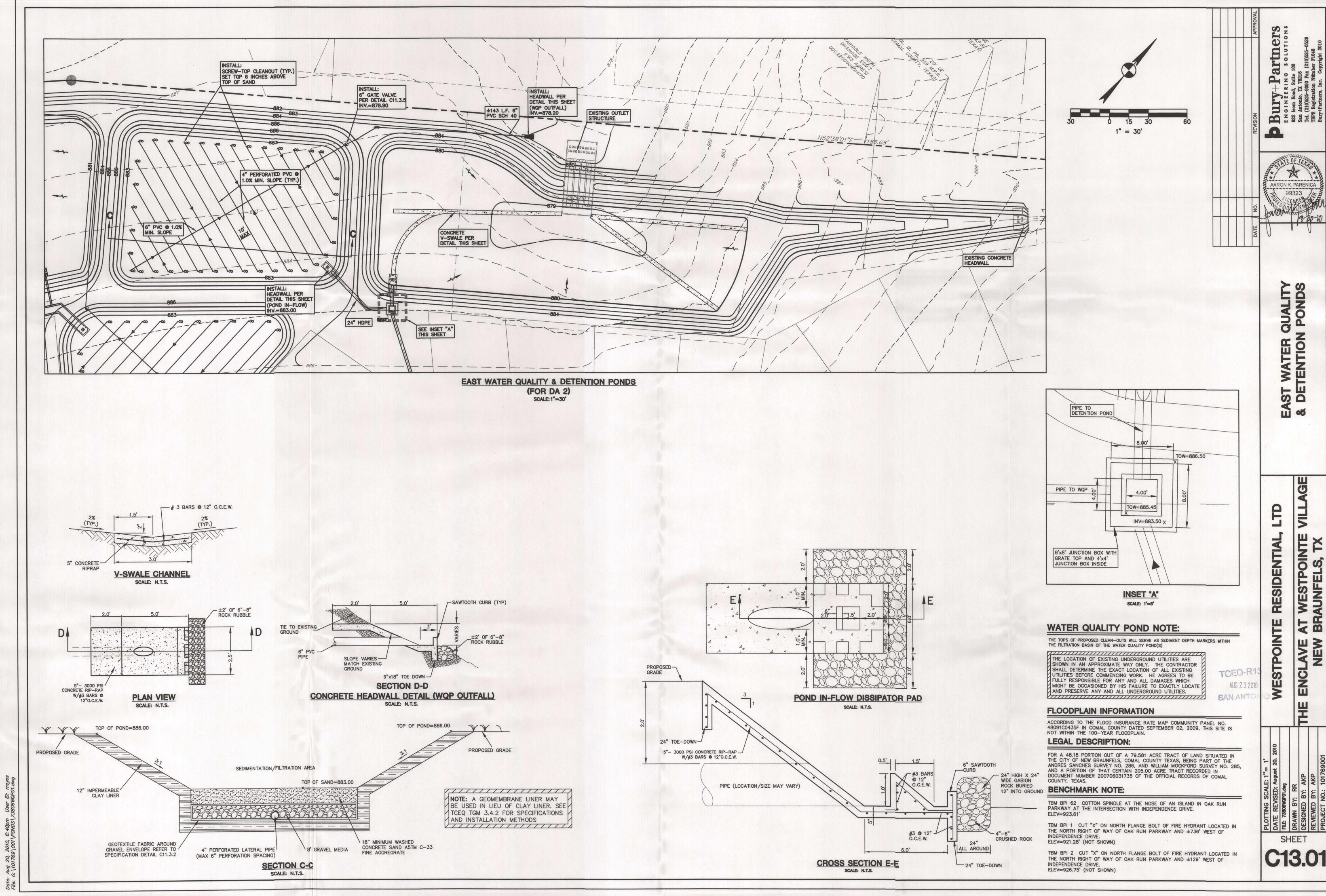
DBury+Partners ENGINEERING SOLUTIONS

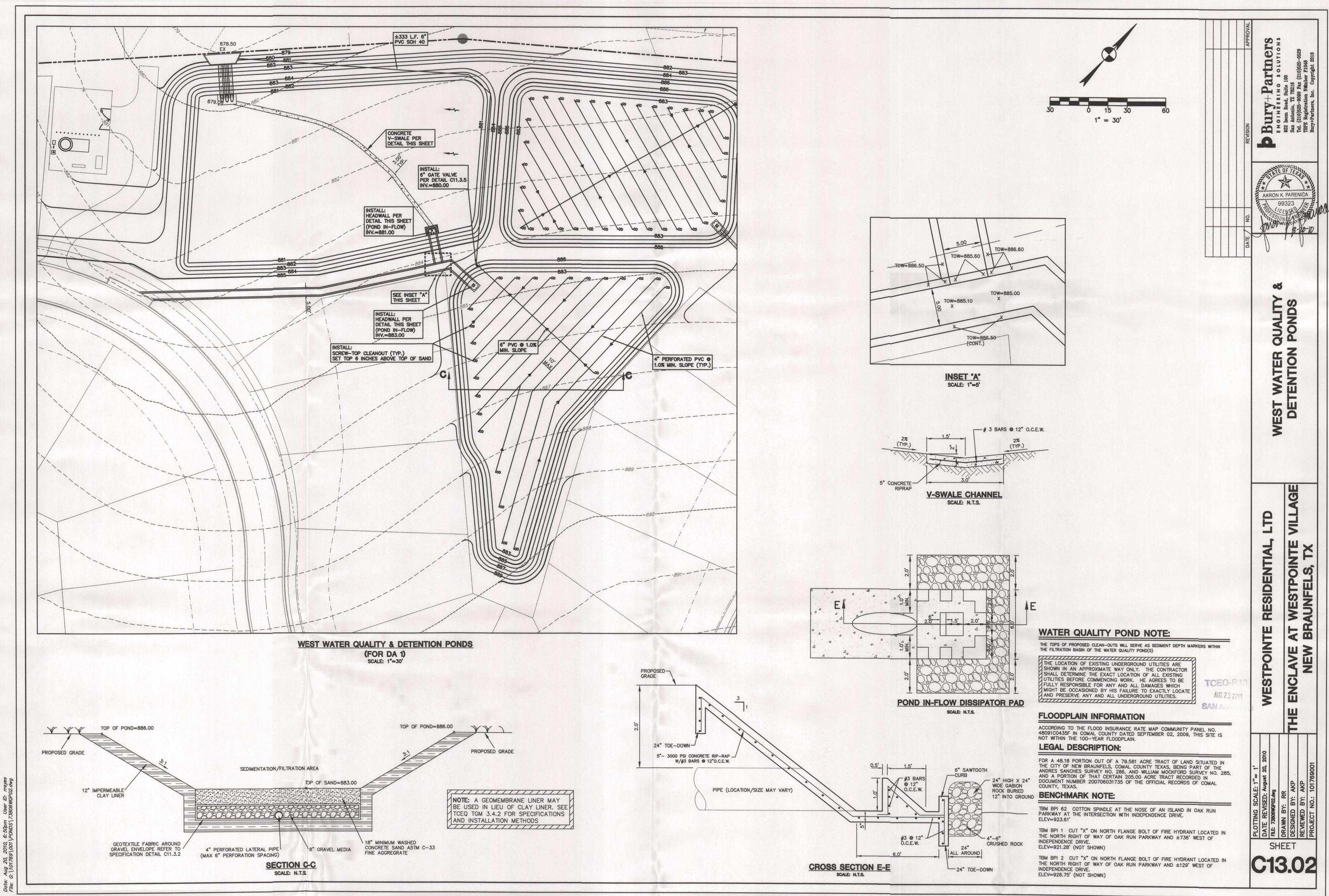
922 Isom Road, Suite 100 San Antonio, TX 78216 Tel. (210)525-9090 Fax (210)525-0529 TBPE Registration Number F1048 Bury+Partners, Inc. Copyright 2010

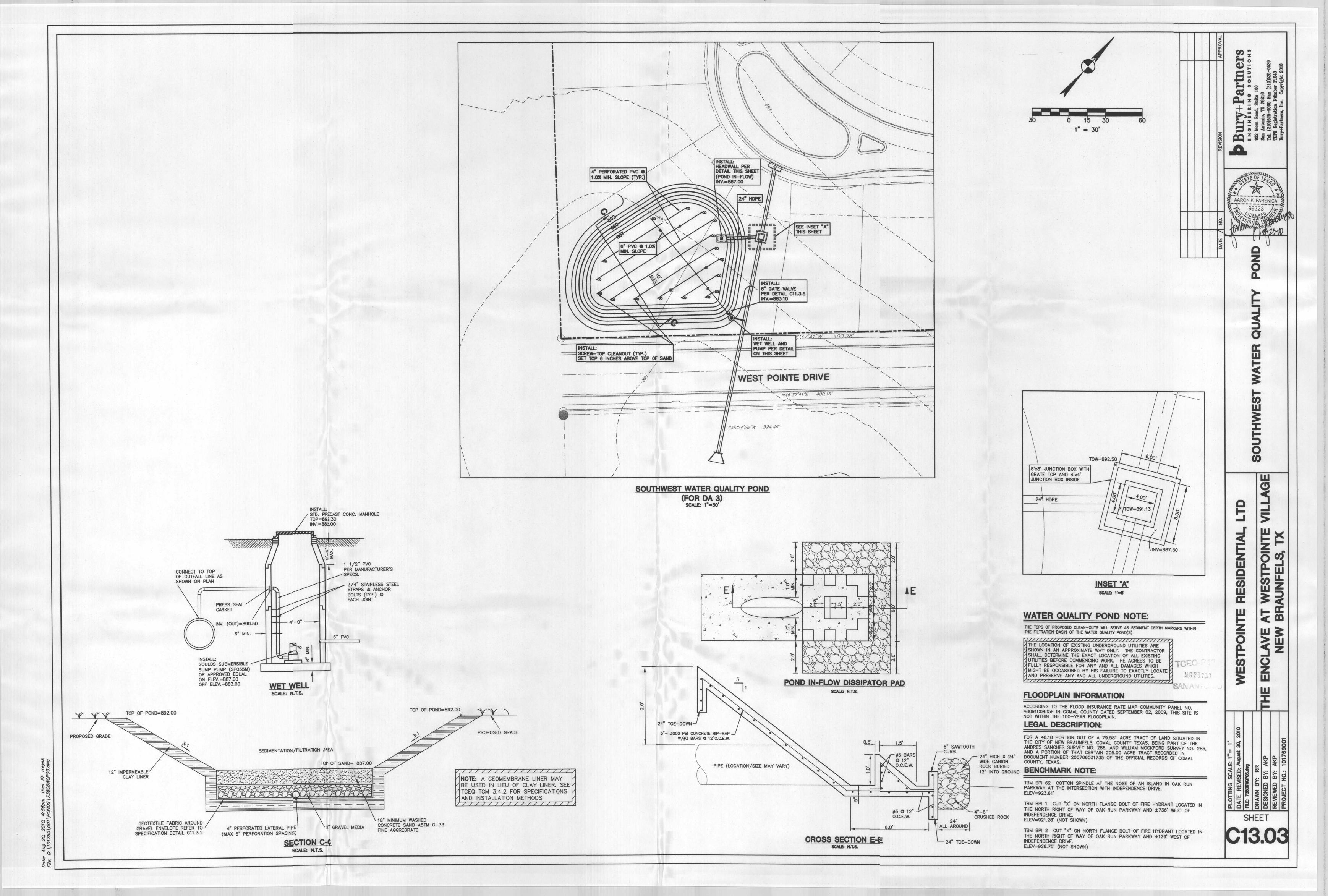
(210) 525-9090

AARON K. PARENICA. P.E. BURY+PARTNERS, INC. 922 ISOM ROAD, SUITE 100 SAN ANTONIO, TEXAS 78216 DATE









TEXAS COMMISSION ON ENVIRONMENTAL QUALITY WATER POLLUTION ABATEMENT **PLAN NOTES:**

1. CONTRACTOR IS ADVISED THAT TCEQ DOES NOT ALLOW CHANGES TO PERMANENT POLLUTION ABATEMENT MEASURES WITHOUT THEIR PRIOR APPROVAL.

. TOPS OF CLEANOUTS SHALL BE SET AT SIX INCHES ABOVE THE SAND 3. SEDIMENT WILL BE REMOVED WHEN THE MATERIAL FILLS THE BASIN TO THE

TOPS OF THE CLEANOUTS.

APPROVED BY ENGINEER.

- OVERFLOW WEIR

4. CONTRACTOR TO HYDROMULCH EARTHEN SLOPES FOR SLOPE STABILIZATION DURING INITIAL BASIN CONSTRUCTION AND MAINTAIN WATERING UNTIL VEGETATION

5. AS AN ALTERNATE TO ITEM 4, CONTRACTOR MAY USE SEED IMPREGNATED STRAW MATTING FOR SLOPE STABILIZATION. MATTING MATERIAL TO BE

6. CONTRACTOR SHALL NOTIFY CERTIFYING ENGINEER WHEN BASIN CONSTRUCTION HAS PROGRESSED TO THE FOLLOWING MILESTONES:

- a.) BASIN LINER IN PLACE AND UNDER DRAIN SYSTEM IS IN PLACE WITHOUT
- b.) GRAVEL AROUND UNDER DRAIN SYSTEM IS IN PLACE AND FILTER FABRIC IS INSTALLED AND ATTACHED TO WALLS OR RIP-RAP.
- c.) SAND FILTER MEDIA HAS BEEN PLACED & BASIN HAS BEEN COMPLETELY FINISHED INCLUDING SOD OR SEED PLACEMENT ON SIDE SLOPES (WHERE APPLICABLE).

7. WORK SHALL NOT CONTINUE ON THE BASIN UNTIL THE ENGINEER HAS HAD AN OPPORTUNITY TO OBSERVE THE STATUS OF CONSTRUCTION AT EACH STAGE. CONTRACTOR SHALL PROVIDE ENGINEER A MINIMUM OF 24 HOURS ADVANCE NOTICE PRIOR TO TIME THE BASIN WILL BE AT THE REQUIRED STAGE.

8. UPON SUBSTANTIAL COMPLETION CONTRACTOR TO PROVIDE CERTIFYING ENGINEER WITH FIELD SHOTS VERIFYING ELEVATIONS OF THE FOLLOWING: - TOP OF BANK AT EACH CORNER OF BASIN - TOE OF SLOPE AT EACH CORNER OF BASIN (INSIDE BASIN TOE) - SPLASH PAD/INLET PIPE

O. CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITY WITH STRUCTURAL ENGINEER SO AS TO PROVIDE APPROPRIATE OPPORTUNITY FOR STRUCTURAL ENGINEER TO MAKE THE NECESSARY CONSTRUCTION OBSERVATIONS DURING

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GENERAL CONSTRUCTION NOTES:

WRITTEN CONSTRUCTION NOTIFICATION MUST BE GIVEN TO THE APPROPRIATE TCEQ REGIONAL OFFICE NO LATER THAN 48 HOURS PRIOR TO COMMENCEMENT OF THE REGULATED ACTIVITY. INFORMATION MUST INCLUDE THE DATE ON WHICH HE REGULATED ACTIVITY WILL COMMENCE, THE NAME OF THE APPROVED PLAN FOR THE REGULATED ACTIVITY, AND THE NAME OF THE PRIME CONTRACTOR AND THE NAME AND TELEPHONE NUMBER OF THE CONTACT PERSON.

. ALL CONTRACTORS CONDUCTING REGULATED ACTIVITIES ASSOCIATED WITH THIS PROJECT MUST BE PROVIDED WITH COMPLETE COPIES OF THE APPROVED WATER POLLUTION ABATEMENT PLAN AND THE TCEQ LETTER INDICATING THE SPECIFIC CONDITIONS OF ITS APPROVAL. DURING THE COURSE OF THESE REGULATED ACTIVITIES, THE CONTRACTORS ARE REQUIRED TO KEEP ON-SITE COPIES OF THE APPROVED PLAN

3. IF ANY SENSITIVE FEATURE IS DISCOVERED DURING CONSTRUCTION, ALL REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MUST BE SUSPENDED IMMEDIATELY. THE APPROPRIATE TCEQ REGIONAL OFFICE MUST BE IMMEDIATELY NOTIFIED OF ANY SENSITIVE FEATURES ENCOUNTERED DURING CONSTRUCTION. THE REGULATED ACTIVITIES NEAR THE SENSITIVE FEATURE MAY NOT PROCEED UNTIL THE TCEQ HAS REVIEWED AND APPROVED THE METHODS PROPOSED TO PROTECT THE SENSITIVE FEATURE AND THE EDWARDS AQUIFER FROM ANY POTENTIALLY ADVERSE IMPACTS TO WATER QUALITY.

4. NO TEMPORARY ABOVEGROUND HYDROCARBON AND HAZARDOUS SUBSTANCE STORAGE TANK SYSTEM IS INSTALLED WITHIN 150 FEET OF A DOMESTIC, INDUSTRIAL, IRRIGATION, OR PUBLIC WATER SUPPLY WELL, OR OTHER SENSITIVE FEATURE.

5. ALL TEMPORARY EROSION AND SEDIMENTATION (E&S) CONTROL MEASURES MUST BE PROPERLY SELECTED, INSTALLED, AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS AND GOOD ENGINEERING PRACTICES. CONTROLS SPECIFIED IN THE TEMPORARY STORM WATER SECTION OF THE APPROVED EDWARDS AQUIFER PROTECTION PLAN ARE REQUIRED DURING CONSTRUCTION. IF INSPECTIONS INDICATE A CONTROL HAS BEEN USED INAPPROPRIATELY, OR INCORRECTLY, THE APPLICANT MUST REPLACE OR MODIFY THE CONTROL FOR SITE SITUATIONS. THE CONTROLS MUST REMAIN IN PLACE UNTIL DISTURBED AREAS ARE REVEGETATED AND THE AREAS HAVE BECOME PERMANENTLY STABILIZED.

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8. LITTER, CONSTRUCTION DEBRIS, AND CONSTRUCTION CHEMICALS EXPOSED TO STORMWATER SHALL BE PREVENTED FROM BECOMING A POLLUTANT SOURCE FOR STORMWATER DISCHARGES (E.G., SCREENING OUTFALLS, PICKED UP DAILY).

9. ALL SPOILS (EXCAVATED MATERIAL) GENERATED FROM THE PROJECT SITE MUST BE STORED ON-SITE WITH PROPER E&S CONTROLS. FOR STORAGE OR DISPOSAL OF SPOILS AT ANOTHER SITE ON THE EDWARDS AQUIFER RECHARGE ZONE, THE OWNER OF THE SITE MUST RECEIVE APPROVAL OF A WATER POLLUTION ABATEMENT PLAN FOR THE PLACEMENT OF FILL MATERIAL OR MASS GRADING PRIOR TO THE PLACEMENT OF SPOILS AT THE OTHER SITE.

10. STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE IN PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT IN NO CASE MORE THAN 14 DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT PORTION OF THE SITE HAS TEMPORARILY OR PERMANENTLY CEASED. WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER CONSTRUCTION ACTIVITY TEMPORARY OR PERMANENTLY CEASE IS PRECLUDED BY WEATHER CONDITIONS, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE. WHERE CONSTRUCTION ACTIVITY ON A PORTION OF THE SITE IS TEMPORARILY CEASED, AND EARTH DISTURBING ACTIVITIES WILL BE RESUMED WITHIN 21 DAYS, TEMPORARY STABILIZATION MEASURES DO NOT HAVE TO BE INITIATED ON THAT PORTION OF SITE. IN AREAS EXPERIENCING DROUGHTS WHERE THE INITIATION OF STABILIZATION MEASURES BY THE 14TH DAY AFTER CONSTRUCTION ACTIVITY HAS TEMPORARILY OR PERMANENTLY CEASED IS PRECLUDED BY SEASONAL ARID CONDITIONS, STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICABLE.

11. THE FOLLOWING RECORDS SHALL BE MAINTAINED AND MADE AVAILABLE TO THE TCEQ UPON REQUEST: THE DATES WHEN MAJOR GRADING ACTIVITIES OCCUR; THE DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE; AND THE DATES WHEN STABILIZATION MEASURES ARE INITIATED. 12. THE HOLDER OF ANY APPROVED EDWARD AQUIFER PROTECTION PLAN MUST NOTIFY THE APPROPRIATE REGIONAL OFFICE IN WRITING AND OBTAIN APPROVAL FROM THE EXECUTIVE DIRECTOR PRIOR TO INITIATING ANY OF THE FOLLOWING:

A. ANY PHYSICAL OR OPERATIONAL MODIFICATION OF ANY WATER POLLUTION ABATEMENT STRUCTURE(S), INCLUDING BUT NOT LIMITED TO PONDS, DAMS,

BERMS, SEWAGE TREATMENT PLANTS, AND DIVERSIONARY STRUCTURES; B. ANY CHANGE IN THE NATURE OR CHARACTER OF THE REGULATED ACTIVITY FROM THAT WHICH WAS ORIGINALLY APPROVED OR A CHANGE WHICH WOULD SIGNIFICANTLY IMPACT THE ABILITY OF THE PLAN TO PREVENT POLLUTION OF THE EDWARDS AQUIFER;

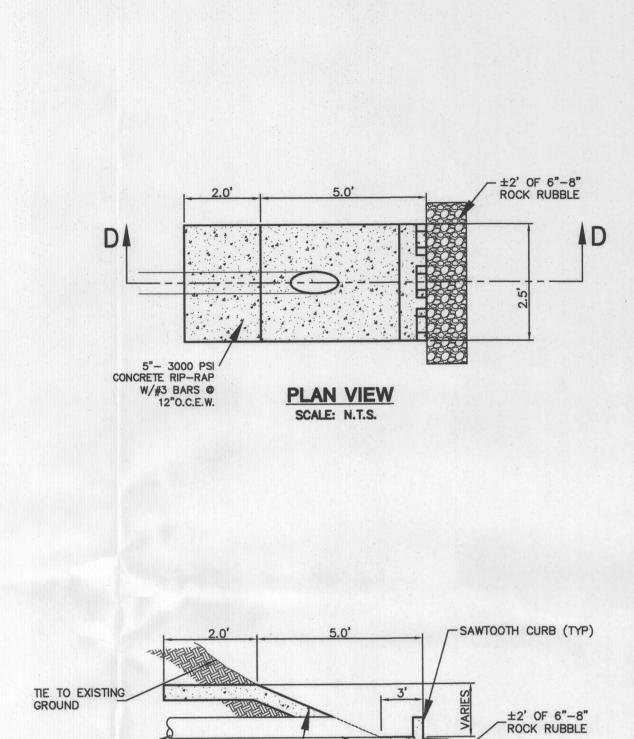
C. ANY DEVELOPMENT OF LAND PREVIOUSLY IDENTIFIED AS UNDEVELOPED IN THE ORIGINAL WATER POLLUTION ABATEMENT PLAN.

SAN ANTONIO REGIONAL OFFICE 14250 JUDSON RD. SAN ANTONIO, TEXAS 78233-4480 PHONE (210) 490-3096 FAX (210) 545-4329

AUSTIN REGIONAL OFFICE 2800 S. IH-35, SUITE 100 AUSTIN, TEXAS 78704-5712 PHONE (512) 339-2929 FAX (512) 339-3795

C11.3.1

C11.3.4



SLOPE VARIES -

9"x18" TOE DOWN -

SECTION D-D

SCALE: N.T.S.

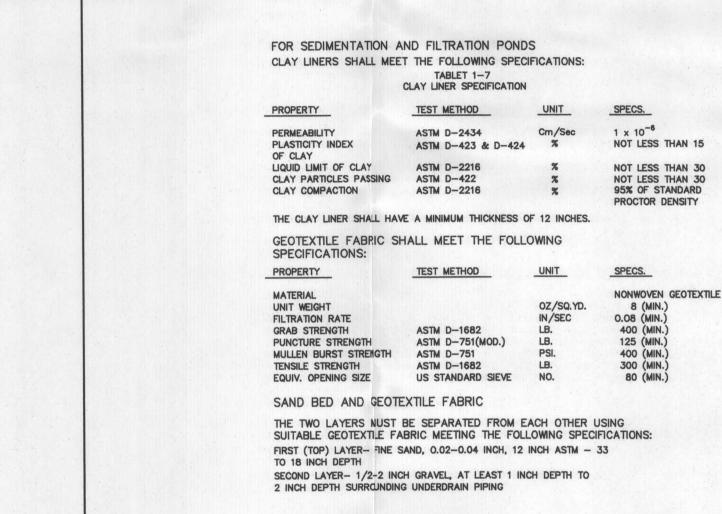
CONCRETE HEADWALL DETAIL (WQP OUTFALL)
SCALE: N.T.S.

NOT USED

SCALE: N.T.S.

MATCH EXISTING

GROUND



PERFORATION LAYOUT

SET 1/2" PERFORATIONS

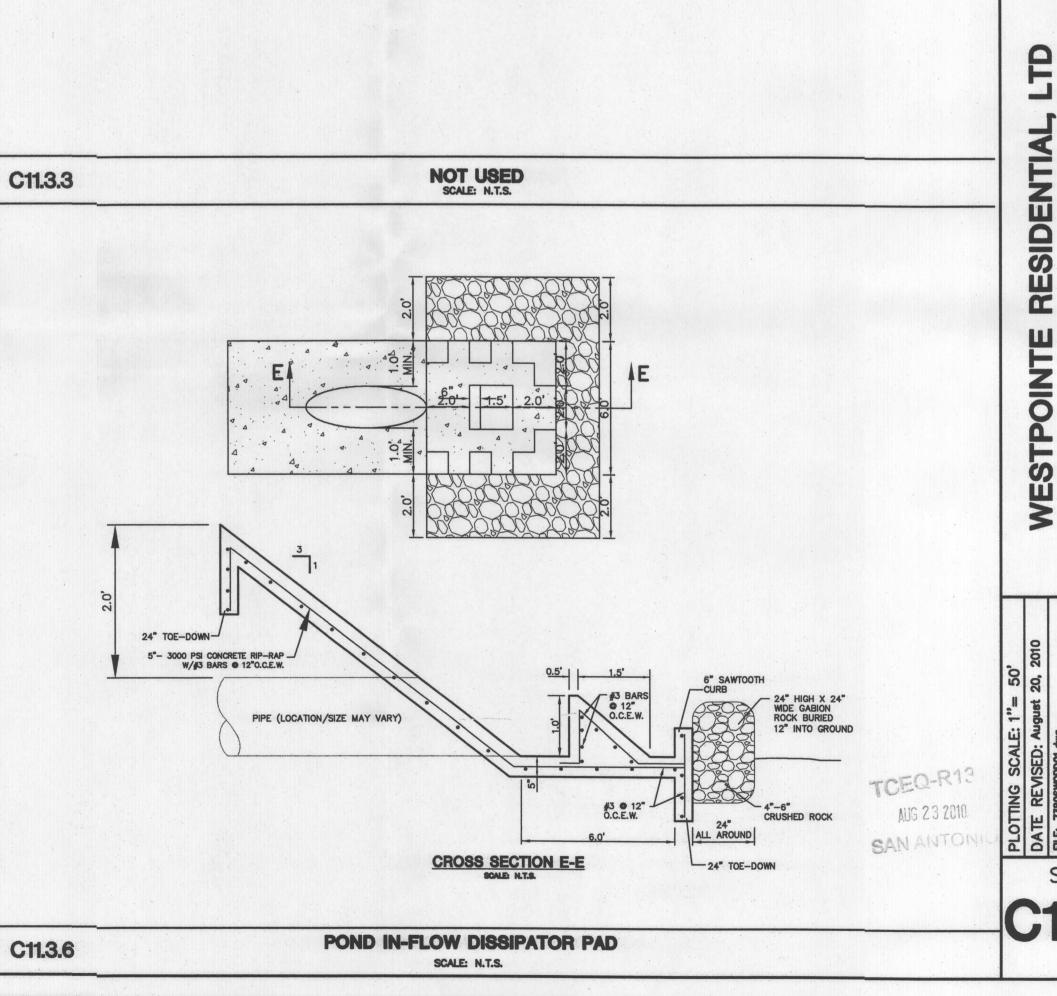
SAND BED ASTM - 33

(top of bed to be horizontal)

10' C/C TYP.

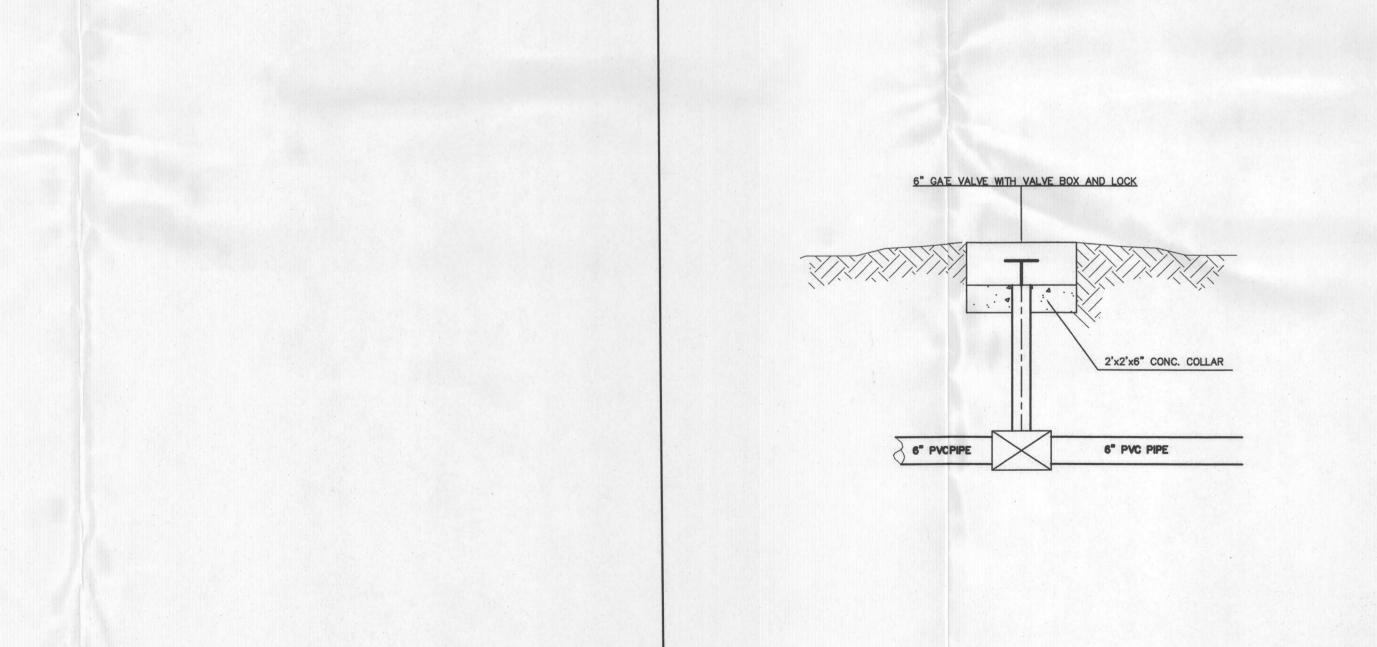
5' FROM EDGE

A. SAND BED PROFILE (with gravel filter)



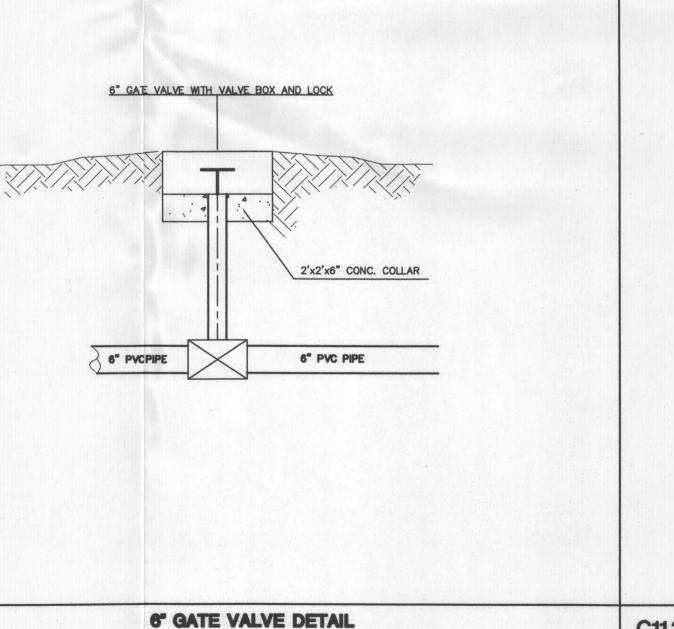
AARON K. PARENICA 99323

OINTE S, TX



C11.3.5

C11.3.2



SCALE: N.T.S.

FILTRATION POND SAND BED W/ GEOTEXTILE FABRIC SCALE: N.T.S.

Texas Commission on Environmental Quality TSS Removal Calculations		
Project Name: The Enclave at Westpointe Village Date Prepared: 8/1/2010		
1. The Required Load Reduction for the total project:		
Calculations from RG-348 Page 3-29 Equation 3.3: $L_M = 27.2(A_N \times P)$ Pages 3-27 to 3-30		
$L_{\text{M TOTAL PROJECT}} = \text{Required TSS removal resulting from the proposed development} = 809$ $A_{\text{N}} = \text{Net increase in impervious area for the project}$ $P = \text{Average annual precipitation, inches}$	% of increase	ed load
Site Data: Determine Required Load Removal Based on the Entire Project County =	Comal	
Total project area included in plan * = Predevelopment impervious area within the limits of the plan * =	53.82	acres
Total post-development impervious area within the limits of the plan* = Total post-development impervious cover fraction * =	31.43 0.58	acres
P =	33	inches
$L_{M TOTAL PROJECT} =$	28212	lbs.
Number of drainage basins / outfalls areas leaving the plan area =	3	
2. Drainage Basin Parameters (This information should be provided for each basin):		
Drainage Basin/Outfall Area No. =	2	
Total drainage basin/outfall area =	20.87	acres
Predevelopment impervious area within drainage basin/outfall area = Post-development impervious area within drainage basin/outfall area =	0.00 10.77	acres
Post-development impervious fraction within drainage basin/outfall area = L _{M THIS BASIN} =	0.52 9667	lbs.
3. Indicate the proposed BMP Code for this basin.		
Proposed BMP = Removal efficiency =	SF 89	abbreviation percent
4. Calculate Maximum TSS Load Removed (L _R) for this Drainage Basin by the selected BMP Type.		Porodit
RG-348 Page 3-33 Equation 3.7: $LR = (BMP \text{ efficiency}) \times P \times (A_1 \times 34.6 + A_P \times 0.54)$		
A_C = Total On-Site drainage area in the BMP catchment area		
$A_{\rm I}$ = Impervious area proposed in the BMP catchment area		
A_P = Pervious area remaining in the BMP catchment area L_R = TSS Load removed from this catchment area by the proposed BMP		
$A_C =$	20.87	acres
$A_{I} =$	10.77	acres
$A_{p} = I_{p}$	10.10	acres
$L_R =$	11105	lbs
	11105	lbs
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area		
	9700 0.87	lbs.
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area $Desired\ L_{M\ THIS\ BASIN} = F =$	9700	
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area $Desired\ L_{M\ THIS\ BASIN} = F = $ $F = $ 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area.	9700	
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area $Desired\ L_{M\ THIS\ BASIN} = F =$	9700	
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient =	9700 •.87 1.44 •.37	lbs.
5. Calculate Fraction of Annual Runoff to Treat the drainage basin / outfall area Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume =	9700 •.87	lbs.
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume =	9700 •.87 1.44 •.37	lbs.
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP =	9700 •.87 1.44 •.37 3)981 •.00 •.00	lbs. inches cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient =	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00	lbs. inches cubic feet acres acres
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area =	9700 0.87 1.44 0.37 3)981 0.00 0.00 0	lbs. inches cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00 0 •.00 0	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) =	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00 •.00 •.00	lbs. inches cubic feet acres acres
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00 0 •.00 0	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Required Water Quality Volume for retention basin =	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00 0 •.00 0	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System	9700 •.87 1.44 •.37 3)981 •.00 •.00 •.00 •.00 •.00 •.00 •.47978	inches cubic feet acres acres cubic feet cubic feet
Desired L _{M This Basin} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations:	9700 0.87 1.44 0.37 3)981 0.00 0 0.00 0 7996 47978	inches cubic feet acres acres cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 0 0 0 0 0 0	inches cubic feet acres acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate =	9700 0.87 1.44 0.37 3)981 0.00 0 0.00 0 7996 47978 NA	inches cubic feet acres acres cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 0 0 0 0 0 0	inches cubic feet acres acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Required Water Quality Volume for extended detention basin =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area off-site area off-site water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 Required Water Quality Volume for extended detention basin = Pages 3-46 to 3-51	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious cover draining to BMP = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 47978 NA NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-46 to 3-51	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA	inches cubic feet acres acres cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site Impervious cover draining to BMP = Impervious cover draining to BMP = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 47978 NA NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Coff-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation Area Calculations: Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Maximum sedimentation basin area (2' Depth) =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 0 0 1.00 0 0 1.00 0 1.00 0 1.00 0 1.00 0 1.00	inches cubic feet acres acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area and off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Minimum filter basin area = Maximum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) =	9700 0.87 1.44 0.37 3)981 0.00 0 0.00 0 7996 47978 NA NA NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Coff-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation Area Calculations: Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for sedimentation basin = Maximum sedimentation basin area (2' Depth) =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA NA NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area. Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area and off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Minimum filter basin area = Maximum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA NA NA NA	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Off-site Impervious cover draining to BMP = Off-site Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Required Water Quality Volume for retention basin = Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Maximum sedimentation basin area (2' Depth) = Minimum filter basin area (2' Depth) = Minimum sedimentation basin area (8' Depth) = Minimum sedimentation basin area (8' Depth) =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 47978 NA NA NA NA NA	inches cubic feet acres cubic feet
Desired L _{M THIS BASIN} = F = 6. Calculate Capture Volume required by the BMP Type for this drainage basin / outfall area Calculations from RG-348 Pages 3-34 to 3-36 Rainfall Depth = Post Development Runoff Coefficient = On-site Water Quality Volume = Calculations from RG-348 Pages 3-36 to 3-37 Off-site area draining to BMP = Impervious cover draining to BMP = Off-site Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Water Quality Volume = Storage for Sediment = Total Capture Volume (required water quality volume(s) x 1.20) = 7. Retention/Irrigation System Designed as Required in RG-348 Pages 3-42 to 3-46 Irrigation Area Calculations: Soil infiltration/permeability rate = Irrigation area = 8. Extended Detention Basin System Designed as Required in RG-348 Pages 3-46 to 3-51 9. Filter area for Sand Filters Designed as Required in RG-348 Pages 3-58 to 3-63 Water Quality Volume for extended detention basin = Maximum sedimentation basin area (2' Depth) = Minimum filter basin area = Maximum sedimentation basin area (2' Depth) = Minimum sedimentation basin area (8' Depth) = 9B. Partial Sedimentation and Filtration System Water Quality Volume for combined basins =	9700 0.87 1.44 0.37 3)981 0.00 0 0 0 0 0 0 0 0 0 0 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1	inches cubic feet acres acres cubic feet cubic feet cubic feet cubic feet cubic feet cubic feet square feet square feet square feet square feet square feet

Texas Commission on Environm TSS Removal Calculations	nental Quality			
Project Nam Date Prepare	e: The Enclave at Westpointe Vill d: 8/1/2010	lage		
1. The Required Load Reduction				
Calculations from RG-348 Pages 3-27 to 3-30	Page 3-29 Equation	3.3: $L_M = 27.2(A_N \times P)$		
$L_{ ext{M}}$ total project $A_{ ext{N}}$	Net increase in impervious area to		% of increas	ed load
	= Average annual precipitation, inc			
Site Dat	a: Determine Required Load Remo	val Based on the Entire Project County =	Comal	
		tal project area included in plan * =	53.82	acres
	Total post-development impervious	area within the limits of the plan * = area within the limits of the plan* =	0.00 31.43	acres
		pment impervious cover fraction * =	0.58	
		P =	33	inches
		$L_{M TOTAL PROJECT} =$	28212	lbs.
		Outfalls areas leaving the plan area =	3	
2. Drainage Basin Parameters (This information should be provide	led for each basin):		
	I	Orainage Basin/Outfall Area No. =	3	
		Total drainage basin/outfall area =	11.74	acres
	Predevelopment impervious area	within drainage basin/outfall area = within drainage basin/outfall area =	0.00 8.27	acres
Po	est-development impervious fraction	within drainage basin/outfall area =	0.70	acres
		$L_{M \text{ THIS BASIN}} =$	7423	lbs.
3. Indicate the proposed BMP (Code for this basin.			
		Proposed BMP = Removal efficiency =	SF 89	abbreviatio
4. Calculate Maximum TSS Loa	d Pamayad (I) for this Drainage	E Basin by the selected BMP Type.	09	percent
4. Carculate Maximum 188 Loa				
	RG-348 Page 3 LR = (BMP efficiency) x	F-33 Equation 3.7: CP x (A _I x 34.6 + A _P x 0.54)		
$egin{array}{c} A_{ m I} \ A_{ m P} \end{array}$	= Total On-Site drainage area in the = Impervious area proposed in the = Pervious area remaining in the B	BMP catchment area MP catchment area		
L_R	= TSS Load removed from this cate	chment area by the proposed BMP		
		$A_C =$	11.74	acres
		$A_{I} =$	8.27	acres
			3.41	acres
		$A_{P} = L_{R} =$	3.41 8458	acres lbs
5. Calculate Fraction of Annual	Runoff to Treat the drainage basi	$A_P = L_R =$		
5. Calculate Fraction of Annual	Runoff to Treat the drainage basi	$A_{P}=$ $L_{R}=$ in / outfall area	8458	lbs
5. Calculate Fraction of Annual	Runoff to Treat the drainage basi	$A_P = L_R =$		
	Runoff to Treat the drainage basi	$\begin{array}{c} A_P = \\ L_R = \end{array}$ $\begin{array}{c} \text{in / outfall area} \\ \\ \text{Desired L_{M THIS BASIN} = $} \\ \\ F = \end{array}$	7500	lbs
		$\begin{array}{c} A_P = \\ L_R = \end{array}$ $\begin{array}{c} \text{in / outfall area} \\ \\ \text{Desired L_{M THIS BASIN} = $} \\ \\ F = \end{array}$	7500	lbs
6. Calculate Capture Volume re	quired by the BMP Type for this	$A_P = L_R = $ $L_R = $ $Desired L_{M THIS BASIN} = F = $ $F = $ $drainage basin / outfall area.$ $Rainfall Depth = $	7500 0.89	lbs
6. Calculate Capture Volume re Calculations from RG-348	quired by the BMP Type for this	$A_{P} = L_{R} = $ $L_{R} = $ $Desired L_{M THIS BASIN} = F = $ $F = $ $drainage basin / outfall area.$ $Rainfall Depth = Development Runoff Coefficient = $	7500 0.89 1.60 0.51	lbs.
6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36	quired by the BMP Type for this	$A_P = L_R = $ $L_R = $ $Desired L_{M THIS BASIN} = F = $ $F = $ $drainage basin / outfall area.$ $Rainfall Depth = $	7500 0.89	lbs.
6. Calculate Capture Volume re Calculations from RG-348	quired by the BMP Type for this	$A_{P} = L_{R} = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	7500 0.89 1.60 0.51 34605	lbs. lbs. inches cubic feet
6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	quired by the BMP Type for this of Post Off-site In	$\begin{array}{c} A_P = \\ L_R = \end{array}$ $\begin{array}{c} \text{Im / outfall area} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	7500 0.89 1.60 0.51 34605	lbs.
6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	quired by the BMP Type for this of Post Off-site In	$A_{P} = \\ L_{R} = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	7500 0.89 1.60 0.51 34605	lbs. lbs. inches cubic feet acres
6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348	quired by the BMP Type for this of Post Off-site In	$\begin{array}{c} A_P = \\ L_R = \end{array}$ $\begin{array}{c} \text{Im / outfall area} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	7500 0.89 1.60 0.51 34605 0.00 0.00	lbs. lbs. inches cubic feet acres
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6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37	quired by the BMP Type for this of Post Off-site In	$\begin{array}{c} A_P = \\ L_R = \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	7500 0.89 1.60 0.51 34605 0.00 0.00 0	lbs. lbs. inches cubic feet acres acres
6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System	quired by the BMP Type for this of Post Off-site In	A _P = L _R = In / outfall area Desired L _{M THIS BASIN} = F = drainage basin / outfall area. Rainfall Depth = Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment =	7500 0.89 1.60 0.51 34605 0.00 0 0.00 0	lbs. lbs. inches cubic feet acres cubic feet
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6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System	quired by the BMP Type for this experience of the Post Off-site In	A _P = L _R = In / outfall area Desired L _{M THIS BASIN} = F = drainage basin / outfall area. Rainfall Depth = Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Water quality volume(s) x 1.20) =	7500 0.89 1.60 0.51 34605 0.00 0 0.00 0 6921 41527	lbs. lbs. inches cubic feet acres cubic feet cubic feet
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6. Calculate Capture Volume re Calculations from RG-348 Pages 3-34 to 3-36 Calculations from RG-348 Pages 3-36 to 3-37 7. Retention/Irrigation System Designed as Required in RG-348	Quired by the BMP Type for this of Post Off-site In In Total Capture Volume (required Required Water Quired Water Quired Water Quired Section 1)	A _P = L _R = In / outfall area Desired L _{M THIS BASIN} = F = drainage basin / outfall area. Rainfall Depth = Development Runoff Coefficient = On-site Water Quality Volume = Off-site area draining to BMP = Impervious cover draining to BMP = Impervious fraction of off-site area = Off-site Runoff Coefficient = Off-site Water Quality Volume = Storage for Sediment = Interval and the water quality volume(s) x 1.20) = Quality Volume for retention basin = Irrigation Area Calculations: Oil infiltration/permeability rate =	7500 0.89 1.60 0.51 34605 0.00 0 0.00 0 6921 41527 NA	lbs. lbs. inches cubic feet acres cubic feet cubic feet cubic feet
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-				
		Water Quality Ponds Sum	mary Tabla	
		Drainage Area:		
		Impervious Coverage:	53.82 Acres 0.00 Acres	
		tion Impervious Coverage:	31.43	
	Net Increase i	n Impervious Coverage:	31.43	
	Total Required	d Removal Load (L _M)	28,212	-
	Total Desired	Load Removed by Ponds:		ICEC
		DA-1	11,100	AUG 2
		DA-2	9,700	SANAN
		DA-3	7,500	
		Total	28,300	
	Surp	lus Load Removal (LBS):	88	
		Pond Sizing		
	DAIR	Filter Area Required	Filter Area Provid	led
	DA-1 Pond DA-2 Pond	4,703 3,998	17,401 18,310	
	DA-3 Pond	3,461	7,366	
	DA-1 Pond	Volume Required (CF) 56,431		CF)
1	DA-1 Pond	47,978	59,486 63,830	
	DA-3 Pond	41,527	50,272	
		145,936	173,588	
		Surplus Volume (CF):	27,652	

DA-1 WATER QUALITY POND (WEST)

DA-2 WATER QUALITY POND

DA-3 WATER QUALITY POND (SOUTHWEST)

SHEET

LTD

WESTPOINTE

ATTACHMENT G

INSPECTION, MAINTENANCE, REPAIR AND RETROFIT PLAN

INSPECTION, MAINTENANCE, REPAIR AND RETROFIT PLAN FOR THE ENCLAVE AT WESTPOINTE VILLAGE NEW BRAUNFELS, TEXAS

The owner of the lot where a sedimentation/filtration basin is located is responsible for the inspection, maintenance, and repair of the water quality pond(s).

- First year of operation. The sand filter BMPs will be inspected on a quarterly basis and after large storms for the first year of operation.
- Inspections. BMP facilities will be inspected at least twice a year (once during or immediately following wet weather) to evaluate facility operation. During each inspection, erosion areas inside and downstream of the BMP will be identified and repaired or re-vegetated immediately. With each inspection, any damage to the structural elements of the system (pipes, concrete drainage structures, retaining walls, etc.) will be identified and repaired immediately. Cracks, voids and undermining will be patched/filled to prevent additional structural damage. Trees and root systems will be removed to prevent growth in cracks and joints that can cause structural damage. The inspections should be carried out with as-built pond plans in hand.
- Sediment Removal. Sediment will be removed from the inlet structure and sedimentation chamber when sediment buildup reaches a depth of 6 inches or when the proper functioning of inlet and outlet structures is impaired. Sediment will be cleared from the inlet structure at least every year and from the sedimentation basin at least every 5 years.
- Media Replacement. Maintenance of the filter media will be performed when the drawdown time exceeds 48 hours. When this occurs, the upper layer of sand will be removed and replaced with new material meeting the original specifications. Any discolored sand will also be removed and replaced. In filters that have been regularly maintained, this will be limited to the top 2 to 3 inches.
- Debris and Litter Removal. Debris and litter that accumulates near the sedimentation basin outlet device will be removed during regular mowing operations and inspections. (Particular attention will be paid to floating debris that can eventually clog the control device or riser.)
- Filter Underdrain. The underdrain piping network will be cleaned to remove any sediment buildup as needed to maintain design drawdown time.

INSPECTION, MAINTENANCE, REPAIR AND RETROFIT PLAN FOR THE ENCLAVE AT WESTPOINTE VILLAGE NEW BRAUNFELS, TEXAS

- *Mowing*. Grass areas in and around sand filters will be mowed *at least twice annually* to limit vegetation height to 18 inches. Vegetation on the pond embankments will be mowed as appropriate to prevent the establishment of woody vegetation.
- *Rock Gabion*. Rock gabion structures, when used, will be removed from pond prior to filter media replacement, cleaned and returned to the original location after the filter media replacement is complete.
- Nuisance Control. Most public agencies surveyed indicate that control of insects, weeds, odors, and algae may be needed in some water quality ponds. Nuisance control is probably the most frequent maintenance item demanded by local residents. If the ponds are properly sized and vegetated, these problems should be rare in water quality ponds except under extremely dry weather conditions. Twice a year, the facility should be evaluated in terms of nuisance control (insects, weeds, odors, algae, etc.). Biological friendly methods of control are preferable to chemical applications.

Non-Routine Maintenance

• Structural Repairs and Replacement. Eventually, the various inlet/outlet and riser works in the water quality basins will deteriorate and must be replaced. Some public works experts have estimated that corrugated metal pipe (CMP) has a useful life of about 25 years, while concrete barrels and risers may last from 50 to 75 years. The actual life depends on the type of soil, pH of runoff, and other factors. Polyvinyl chloride (PVC) pipe is a corrosion resistant alternative to metal and concrete pipes. Structural repair and/or replacement may be necessary for any structural objects with signs of corrosion or loss of structural integrity.

J.L. Guerra Jr.	
Name of Owner/Agent	
Signature of Owner/Agent	8/12/2010 Date

ATTACHMENT H

PILOT-SCALE FIELD TESTING PLAN (Not Applicable)

ATTACHMENT I

MEASURES FOR MINIMIZING SURFACE STREAM CONTAMINATION (Not Applicable)

AUTHORIZATION AND APPLICATION FORMS

Agent Authorization Form

For Required Signature
Edwards Aquifer Protection Program
Relating to 30 TAC Chapter 213
Effective June 1, 1999

l	J.L. Guerra, Jr.	
	Print Name	
	Dartner	
	Partner Partner	
	Title - Owner/President/Other	
	Montaginta Decidential LTD	
of	Westpointe Residential, LTD	
	Corporation/Partnership/Entity Name	
t	Acces I/ Devenies D.F.	
have authorized	Aaron K. Parenica, P.E.	***************************************
	Print Name of Agent/Engineer	
of	Bury+Partners	
	Print Name of Firm	

to represent and act on the behalf of the above named Corporation, Partnership, or Entity for the purpose of preparing and submitting this plan application to the Texas Commission on Environmental Quality (TCEQ) for the review and approval consideration of regulated activities.

I also understand that:

- 1. The applicant is responsible for compliance with 30 Texas Administrative Code Chapter 213 and any condition of the TCEQ's approval letter. The TCEQ is authorized to assess administrative penalties of up to \$10,000 per day per violation.
- 2. For those submitting an application who are not the property owner, but who have the right to control and possess the property, additional authorization is required from the owner.
- 3. Application fees are due and payable at the time the application is submitted. The application fee must be sent to the TCEQ cashier or to the appropriate regional office. The application will not be considered until the correct fee is received by the commission.
- 4. A notarized copy of the Agent Authorization Form must be provided for the person preparing the application, and this form must accompany the completed application.
- 5. No person shall commence any regulated activity on the Edwards Aquifer Recharge Zone, Contributing Zone or Transition Zone until the appropriate application for the activity has been filed with and approved by the Executive Director.

SIGNATURE PAGE:

X Aun D	8/6/2010
Applicant's Signature	Date

THE STATE OF <u>TEXAS</u> §

County of BEXAR §

GIVEN under my hand and seal of office on this of day of august 2010

NOTARY PUBLIC

Melanie A. Pierce

Typed or Printed Name of Notary

MY COMMISSION EXPIRES: 2-16-2013

Texas Commission on Environmental Quality Edwards Aquifer Protection Program Application Fee Form

REGULATED ENTITY LOCATION:Southside of Oa		
NAME OF CUSTOMER: Westpointe Residential, CONTACT PERSON: Aaron Parenica or Steve Lin (w/ (Please Print)	LTD Bury+Partners) PHONE: (2	10) 525-9090
Customer Reference Number (if issued): CN	(nine	e digits)
Regulated Entity Reference Number (if issued): RN	(nine	e digits)
Austin Regional Office (3373)	Travis Williamson	
San Antonio Regional Office (3362) 🔲 Bexar 🛛	Comal Medina	Kinney Uvalde
Application fees must be paid by check, certified check, Environmental Quality . Your canceled check will serv your fee payment . This payment is being submitted to	e as your receipt. This form	
Austin Regional Office	San Antonio Regional O	ffice
Mailed to TCEQ: TCEQ – Cashier Revenues Section Mail Code 214 P.O. Box 13088 Austin, TX 78711-3088 Site Location (Check All That Apply): Recharge Zo	Overnight Delivery to TO TCEQ - Cashier 12100 Park 35 Circle Building A, 3rd Floor Austin, TX 78753 512/239-0347	CEQ : ☐ Transition Zone
Type of Plan	Size	Fee Due
Water Pollution Abatement Plan, Contributing Zone Plan: One Single Family Residential Dwelling	Acres	\$
Water Pollution Abatement Plan, Contributing Zone Plan: Multiple Single Family Residential and Parks	48.18 Acres	\$6,500
Water Pollution Abatement Plan, Contributing Zone Plan; Non-residential	Acres	\$
Sewage Collection System	L.F.	\$
Lift Stations without sewer lines	Acres	\$
Underground or Aboveground Storage Tank Facility	Tanks	\$
Piping System(s)(only)	Each	\$
Exception	Each	\$
Extension of Time	Each	\$

8-23-2010 Signature Date

If you have questions on how to fill out this form or about the Edwards Aquifer protection program, please contact us at 210/490-3096 for projects located in the San Antonio Region or 512/339-2929 for projects located in the Austin Region.

Individuals are entitled to request and review their personal information that the agency gathers on its forms. They may also have any errors in their information corrected. To review such information, contact us at 512/239-3282.

Westpointe Residential Ltd

Vendor No: TCEQ01 / Name: TEXAS COMMISSION

1030

Invoice Ref 081710WPAP WPAP Inv Date 08/17/10

Inv Amt 6500.00 Discount 0.00

Adj Amt 0.00 Amt Paid 6500.00

Acct:

Check Date 08/18/10

Total

6500.00

1030

Westpointe Residential Ltd

325 Brown Street Petersburg, VA 23803 5an Antonio, TX IBC Voice - (210) 518-2525 30-1328-1140

1030

***Six Thousand Five Hundred & No/100 Dollars

DATE

AMOUNT

08/18/10

\$6,500.00

PAY TO THE ORDER OF

TEXAS COMMISSION

ON ENVIRONMENTAL QUALITY

Mark Lynnizer SIGNATURE

Security features. Details on back.

Texas Commission on Environmental Quality Edwards Aquifer Protection Program Application Fee Schedule 30 TAC Chapter 213 (effective 05/01/2008)

Water Pollution Abatement Plans and Modifications
Contributing Zone Plans and Modifications

PROJECT	PROJECT AREA IN ACRES	FEE
One Single Family Residential Dwelling	< 5	\$650
Multiple Single Family Residential and Parks	< 5 5 < 10 10 < 40 40 < 100 100 < 500 ≥ 500	\$1,500 \$3,000 \$4,000 \$6,500 \$8,000 \$10,000
Non-residential (Commercial, industrial, institutional, multi-family residential, schools, and other sites where regulated activities will occur)	< 1 1 < 5 5 < 10 10 < 40 40 < 100 ≥ 100	\$3,000 \$4,000 \$5,000 \$6,500 \$8,000 \$10,000

Organized Sewage Collection Systems and Modifications

PROJECT	COST PER LINEAR FOOT	MINIMUM FEE MAXIMUM FEE
Sewage Collection Systems	\$0.50	\$650 - \$6,500

Underground and Aboveground Storage Tank System Facility Plans and Modifications

PROJECT	COST PER TANK OR PIPING SYSTEM	MINIMUM FEE MAXIMUM FEE
Underground and Aboveground Storage Tank Facility	\$650	\$650 - \$6,500

Exception Requests

PROJECT	FEE
Exception Request	\$500

Extension of Time Requests

PROJECT	FEE	
Extension of Time Request	\$150	